

## Floodplain Development Permit

## Doddridge County, WV Floodplain Management

This permit gives approval for the development/ project listed that impacts the FEMA-designated floodplain and/or floodway of Doddridge County, WV, pursuant to the rules and regulations established by all applicable Federal, State and local laws and ordinances, including the Doddridge County Floodplain Ordinance. This permit must be posted at the site of work as to be clearly visible and must remain posted during entirety of development.

**Permit #: 18-511** 

Date Approved: May 7, 2018 Expires: May 7, 2019

Issued to: Dominion Energy Transmission, Inc. POC: Joshua Cook

Company Address: 925 White Oak Blvd. Bridgeport, WV 26330

Project Address: Tonkin Station Rd. West Union, WV 26456

Firm: 54017C0120C Lat/Long: 39.311925 N, -80.781238 W

Purpose of development: Bridge Replacement

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Issued by: George Q. Eidel, Doddridge County FPM (or designee)

Date: May 7, 2018

1450

7017

A STATE OF THE STA	
SENDER: COMPLETE THIS SECTION  Complete items 1, 2, and 3.  Print your name and address on the reverse so that we can return the card to you.  Attach this card to the back of the mailpiece, or on the front if space permits.  Nelson Bruffy 309 Tonkin Station Road West Union, WV 26456	A. Signature  X. G. J. J. J. J. Agent  Addressee  B. Received by (Printed Name)  D. Is delivery address different from item 1?  Yes  If YES, enter delivery address below:
9590 9402 2228 6193 5131 32  2. Article Number ( <i>Transfer from service label</i> )	3. Service Type
PS Form <b>3811</b> , July 2015 PSN 7530-02-000-9053	Domestic Return Receipt





First-Class Mail Postage & Fees Paid USPS Permit No. G-10

United States Postal Service

• Sender: Please print your name, address, and ZIP+4® in this box•

Doddridge County OEM/CFM George Eidel 105 Court Street, Suite 3 West Union, WV 26456

<u> 18-511</u>

## FLOODPLAIN PERMIT #18-511

## Dominion Energy Bridge Replacement Tonkin Station Rd and Rt 18 N

TASK	COMPLETE (DATE)	NOTES
CHECK RECEIVED		
US ARMY CORP. ENGINEERS		
(USACE)		
US FISH & WILDLIFE	E /4/2019	
SERVICES (USFWS)	5/4/2018	
WV DEPT. NATURAL		
RESOURCES (WVDNR)		
WV DEPT. ENVIROMENTAL		
PROTECTION (WVDEP)		
STATE HISTORIC &		
PRESERVATION OFFICE		
(SHPO)		
OFFICE of LAND & STREAM		
(OLS)		
DATE OF COMMISSION		
READING	May 15, 2018	
DATE AVAILABLE TO BE		
GRANTED		
PERMIT GRANTED		
COMPLETE		

7017 1450 0001 5869 5605

7017 1450 0001 5869 5612

PNC BANK, N.A. Pittsburgh, PA

8-9/430

CHECK DATE 5/3/2018

PAY Seven Thousand Five Hundred and 00/100 Dollars



AMOUNT 7.500.00

TO Doddridge County Commission





AUTHORIZED SIGNATURE

# 139583# 1:0430000961: 0002272405#

### CIVIL & ENVIRONMENTAL CONSULTANTS, INC.

139583

Check Date: 5/3/2018

Invoice Number	Date	Voucher	Amount	Discounts	Previous Pay	Net Amount
05032018	5/3/2018	000000312533	7,500.00			7,500.00
Doddridge County Commiss	sion	TOTAL	7,500.00			7,500.00
- Operating Account	1	11261				

FP#18-511

COPY

COPY

Check Date:

5/3/2018

			T		Dravious Boy	Net Amount
Invoice Number	Date	l Voucher l	Amount	Discounts	Previous Pay	
		000000040500	7,500.00			7,500.00
05032018	5/3/2018	000000312533				7,500.00
Doddridge County Comm	nission	TOTAL	7,500.00			1,000.00
- Operating Account	1	11261				

FP#18-511

## Doddridge County, West Virginia

RECEIPT NO;	849	DATE: 2018/0	5/07
FR.	OM: CIVIL & ENVIRONMENTAL CONS		7,500.00
SEVEN THOUS.	AND FIVE HUNDRED DOLLARS AND 0	0 CENTS	
F	OR: #18-511 FLOODPLAIN		
00000139583	3 FP-BUILDING PERMITS	020-318	TOTAL: \$7,500.00
	EL HEADLEY ERIFF & TREASURER	<del></del>	MEC LERK

**Customer Copy** 

Deluxe Corporation 1-800-328-0304 or www.deluxe.com/shop



## **Doddridge County Floodplain Permits**

(Week of May 7, 2018)

Please take notice that on the 4<sup>th</sup> day of May, 2018, Dominion Energy filed an application for a Floodplain Permit (#18-511) to develop land located at or about Tonkin Station Rd and Rt 18 N Coordinates 39.311925 N, -80.781238 W. The Application is on file with the Doddridge County Floodplain Manager's Office and may be inspected or copied during regular business hours. Any interested persons who desire to comment shall present the same in writing by June 4, 2018 (20 calendar days after the announcement at the regularly scheduled Doddridge County Commission Meeting) delivered to the Clerk of the County Court at 108 Court Street Ste. 1, West Union, WV 26456. This project is a bridge replacement



May 1, 2018

Doddridge County Commission Attn: George Eidel, Doddridge County Floodplain Manager 118 East Court Street, Room 102 West Union, WV 26456

Subject:

Dominion Energy Transmission, Inc.

LL Tonkin Compressor Station Bridge

No-Rise Certificate

Doddridge County, West Virginia

CEC Project 171-658

Civil & Environmental Consultants, Inc. (CEC) is pleased to evaluate the potential floodplain impacts for the above referenced project on behalf of Dominion Energy Transmission, Inc., 925 White Oaks Blvd, Bridgeport, WV 26330. The proposed work includes replacing the superstructure on the existing permanent bridge over Middle Island Creek and removing the temporary bridge that is currently in place. The project is located in the Middle Island Creek Zone AE Flood Hazard area, according to the Flood Insurance Rate Map (FIRM) for Doddridge County, map panel 54017C0120C with a map revised date of October 4, 2011. Based on the previous hydraulic study for the project area, the 100-year water surface elevation at the bridge location is 768.86'. The low beam of the proposed superstructure will remain the same as the existing conditions, with a low beam elevation of 773.41'. The freeboard between the low beam elevation of the bridge and the 100-year water surface elevation is 4.55'. Therefore, the proposed work will not have an adverse impact on the existing base flood elevation of Middle Island Creek.

This no-rise certificate is provided in support of the floodplain development permit application. Your time and effort in reviewing this floodplain development permit application is appreciated. Please feel free to contact me at 304-933-3119 or via e-mail at <a href="mailto:glinder@cecinc.com">glinder@cecinc.com</a> or contact Mr. Joshua Cook at 681-842-3908 or via e-mail at <a href="mailto:joshua.d.cook@dom.com">joshua.d.cook@dom.com</a> if you have questions or need additional information.

Respectfully submitted,

CIVIL & ENVIRONMENTAL CONSULTANTS, INC.

Andrew P. Darnell, E.I.T.

Assistant Project Manager

Greg S. Linder, P.E.

Principal

## **Doddridge County Floodplain Application Fee Calculation**

Project Construction Cost = \$1,400,000

Base Floodplain Application Fee = \$1,000

Additional Application Fee = 
$$(\$1,400,000 - \$100,000) * (\frac{\$5.00}{\$1,000}) = \$6,500$$

Total Application Fee = \$7,500



Permit# 18-5//
TONKIN Station
Project Name: Bridge Replacement

## Doddridge County, WV

# Floodplain Development Permit Application

This document is to be used for projects that impact/potentially impact the FEMA---designated floodplain and/or floodway of Doddridge County, WV pursuant to the rules and regulations established by all applicable Federal, State and local laws and ordinances, including the Doddridge County Floodplain Ordinance.

## SECTION 1: GENERAL PROVISIONS (APPLICANT TO READ AND SIGN)

- 1. No work may start until a permit is issued.
- 2. The permit may be revoked if any false statements are made herein.
- 3. If revoked, all work must cease until permit is re-issued.
- 4. Development shall not be used or occupied until a Certificate of Compliance is issued.
- 5. The permit will expire if no work is commenced within six months of issuance.
- 6. Applicant is hereby informed that other permits may be required to fulfill local, state, and federal requirements.
- 7. Applicant hereby gives consent to the Floodplain Administrator/Manager or his/her representative to make inspections to verify compliance.
- 8. I THE APPLICANT CERTIFY THAT ALL STATEMENTS HEREIN AND IN ATTACHMENTS TO THIS APPLICATION ARE, TO THE BEST OF MY KNOWLEDGE, TRUE AND ACCURATE.

APPLICANT'S SIGNATURE_	75. F.	
	0-5-0-5-15	

## **Applicant Information:**

Please provide all pertinent data.

Applicant Information		
Responsible Company Name: Dominion Energy	Transmission	, Inc.
Corporate Mailing Address: 925 White Oaks Bl	vd	
City: Bridgeport	State: WV	Zip: 26330
Corporate Point of Contact (POC): Joshua Cook		
Corporate POC Title: Engineer II – Clarksburg,	wv	
Corporate POC Primary Phone: (681) 842-3800	)	
Corporate POC Primary Email: joshua.d.cook@	dom.com	
Corporate FEIN:	Corporate DU	NS:
Corporate Website: https://www.dominioner	ergy.com/	
Local Mailing Address: 925 White Oaks Blvd		
City: Bridgeport	State: WV	Zip: 26330
Local Project Manager (PM): Joshua Cook		
Local PM Primary Phone: (681) 842-3908		
Local PM Secondary Phone: (304) 203-0881		
Local PM Primary Email: joshua.d.cook@dom.o	com	
Person Filing Application: Gregory S. Linder		
Applicant Title: Principal		
Applicant Primary Phone: (304) 848-7104		
Applicant Secondary Phone: (304) 629-9074		
Applicant Primary Email: glinder@cecinc.com		

## **Project Narrative:**

Describe in detail the proposed development including project name/title, type of development, estimated start and completion timeline, and its potential impact on the floodplain. Use additional copies of this page as needed.

## **Proposed Development:**

Please check all elements of the proposed project that apply.

## **DESCRIPTION OF WORK (CHECK ALL APPLICABLE BOXES)**

## A. STRUCTURAL DEVELOPMENT

	<u>AC</u>	TIVITY				<b>STRUCTU</b>	RAL TYPE
[]	New Struct	ture			[]	Residential	(1 – 4 Family)
[]	Addition				[]	Residential	(more than 4 Family)
[]	Alteration				[]	Non-reside	ntial (floodproofing)
[]	Relocation				[]	Combined I	Jse (res. & com.)
[]	Demolition	1			[]	Replaceme	nt
[]	Manufacti	ured/Mo	bil Home				
В.	OTHER DE	VELOPI	LMENT ACTI	VITIES:			
[]	Fill	[]	Mining	[]	Drilling	: []	Pipelining
[]	Grading						
[]	Excavation	(except	for STRUCTUR	RAL DEVE	LOPMENT	Γ checked at	oove)
[]	Watercour	se Altera	ition (includin	g dredgir	ng and cha	annel modifi	cation)
[]	Drainage Ir	mproven	nents (includir	ng culver	t work)		
[X]	Road, Stree	et, or Bri	dge Construct	ion			
[]	Subdivision	n (includi	ng new expan	sion)			
[]	Individual \	Water or	Sewer Systen	n			
[]	Other (plea	ase speci	fy)				
<u></u>							

## **Development Site/Property Information:**

Please provide physical description of the site/property, along with pertinent ownership (surface and mineral rights) data as applicable. Attach appropriate maps from the WV Flood Tool showing location of proposed development. Use additional copies of this page if development spans multiple property boundaries. Designate each property by number (i.e. Property 1 of 1, Property 2 of 7, etc.)

Property Designation:1_ of	_1_				
Site/Property Information:	- Jan di Milanda				
Legal Description: M I CREEK 2			The second secon	30 F 17 17	and the state of t
Physical Address/911 Address	: MIDDI	EBURG S	TART RT 18 MII	DD	
Decimal Latitude/Longitude: 3	39.3119	25, -80.7	81238		
DMS Latitude/Longitude: 39°1	8′42.93′	'N, 80°46	'52.38"W		
District: 08	Мар:	7		Parcel:	22
Land Book Description:	•				
Deed Book Reference: Deed Bo	ook 006	5, Page 0	440		
Tax Map Reference: 09 08 000	7 0022 (	000 000	0		
Evicting Duildings /Use of Dyoney	etre Come	n mosson C	tation Assassance		
Existing Buildings/Use of Proper	ty: Com	pressor s		10 	
Floodplain Location Data: (to b	e comple	eted by Fl	oodplain Manag	er or des	ignee)
Community: 54017C0120C Num	mber:		Panel:		Suffix:
Location (Lat/Long):		•	Approximate I	Elevatior	ı:
See Attached			Estimated BFE	:	
Is the development in the flood	way?			ment in (	the floodplain?
□ <sub>Yes</sub> □ <sub>No</sub>			Yes	$\bigcirc_{No}$	Zone:
Notes:					

## **Property Owner Data:**

Please provide data on current site/property landowner(s), both surface and mineral rights (as applicable). Use additional copies of this page as needed. Designate each page in relation to each property listed above.

Property Owner Data:		
Name of Primary Owner (PO): H	lope Natural Gas Company c	o Dominion Transmission Inc.
PO Address: 625 LIBERTY AV	MAIL DROP 18-01	
City: PITTSBURGH	State: PA	Zip: 15221
PO Primary Phone:		
PO Secondary Phone:		
PO Primary Email:		
Surface Rights Owner Data:		
Name of Primary Owner (PO):		
PO Address:		
City:	State:	Zip:
=		I -
PO Primary Phone:		
PO Primary Phone: PO Secondary Phone:		
PO Secondary Phone:		
PO Secondary Phone:	Applicable)	
PO Secondary Phone: PO Primary Email:	Applicable)	
PO Secondary Phone: PO Primary Email: Mineral Rights Owner Data: (As	Applicable)	
PO Secondary Phone: PO Primary Email: Mineral Rights Owner Data: (As Name of Primary Owner (PO):	Applicable)  State:	Zip:
PO Secondary Phone: PO Primary Email: Mineral Rights Owner Data: (As Name of Primary Owner (PO): PO Address:		
PO Secondary Phone: PO Primary Email: Mineral Rights Owner Data: (As Name of Primary Owner (PO): PO Address: City:		

## **Contractor Data:**

**Property Designation:** 

of\_

Contractor/Sub-Contractor (C/SC) Information:

Please provide all pertinent data for contractors and sub---contractors that may be participating in this project. Use additional copies of this page as needed. Designate each page in relation to each property listed above.

C/SC Company Name:		
C/SC WV License Number:		
C/SC FEIN:	C/SC DUNS:	
Local C/SC Point of Contact (POC):		
Local C/SC POC Title:		
C/SC Mailing Address:		
City:	State:	Zip-Code:
Local C/SC Office Phone:		
Local C/SC POC Phone:		
Local C/SC POC E-Mail:	-	
Engineer Firm Information:		
Engineer Firm Information: Engineer Firm Name:		
Engineer Firm Name:	Engineer Firr	
Engineer Firm Name: Engineer WV License Number:		
Engineer Firm Name: Engineer WV License Number: Engineer Firm FEIN:		
Engineer Firm Name: Engineer WV License Number: Engineer Firm FEIN: Engineer Firm Primary Point of Contact (POC):		
Engineer Firm Name: Engineer WV License Number: Engineer Firm FEIN: Engineer Firm Primary Point of Contact (POC): Engineer Firm Primary POC Title:		
Engineer Firm Name:  Engineer WV License Number:  Engineer Firm FEIN:  Engineer Firm Primary Point of Contact (POC):  Engineer Firm Primary POC Title:  Engineer Firm Mailing Address:	Engineer Firr	m DUNS:
Engineer Firm Name: Engineer WV License Number: Engineer Firm FEIN: Engineer Firm Primary Point of Contact (POC): Engineer Firm Primary POC Title: Engineer Firm Mailing Address: City:	Engineer Firr	m DUNS:
Engineer Firm Name: Engineer WV License Number: Engineer Firm FEIN: Engineer Firm Primary Point of Contact (POC): Engineer Firm Primary POC Title: Engineer Firm Mailing Address: City: Engineer Firm Office Phone:	Engineer Firr	m DUNS:

## Adjacent and/or Affected Landowners Data

Please provide data for all adjacent and/or affected surface owners (both up and down stream) whose property may be impacted by proposed development as demonstrated by a floodplain study or survey. Use additional copies of this page as needed.

Name of Primary Owner (PO): NELSON BRUFFY ET UX  Physical Address: 309 TONKIN STATION ROAD  City: WEST UNION  State: WV	Zip: 26456
	Zip: 26456
City: WEST UNION State: WV	Zip: 26456
PO Primary Phone:	
PO Secondary Phone:	
PO Primary Email:	
Adjacent Property Owner Data: Upstream	
Name of Primary Owner (PO): CNG TRANSMISSION CORP, C	O DOMINION TRANSMISSION
Physical Address: 625 LIBERTY AV MAIL DROP 18-01	
City: PITTSBURGH State: PA	Zip: 15221
PO Primary Phone:	
PO Secondary Phone:	
PO Primary Email:	
L ,	
Adjacent Property Owner Data: Downstream	
Name of Primary Owner (PO): NELSON BRUFFY ET UX	
Physical Address: 309 TONKIN STATION ROAD	
City: WEST UNION State: WV	Zip: 26456
PO Primary Phone:	
PO Secondary Phone:	
PO Primary Email:	
Adjacent Property Owner Data: Downstream  Name of Primary Owner (PO): CNG TRANSMISSION CORP, C/	O DOMINION TO ANEMISSION
	O DOMINION I RANSMISSION
Physical Address: 625 LIBERTY AV MAIL DROP 18-01	
City: PITTSBURGH State: PA	Zip: 15221
PO Primary Phone:	
PO Secondary Phone:	
PO Primary Email:	

#### Site Plan

A Site Plan is an accurate and detailed map of the proposed development for this project. It shows the size, shape, location and special features of the project property, and the size and location of any development planned to the property, especially as that development will impact the floodplain and/or floodway. Site plans show what currently exists on the project property, and any changes or improvements you are proposing to make. A certified and licensed engineering firm should complete site plans.

#### A SITE PLAN MUST CONTAIN THE FOLLOWING INFORMATION:

- 1. Legal description of the parcel, north arrow and scale
- 2. All property lines and their dimensions
- 3. Names of adjacent roads, location of driveways
- Location of sloughs, tributaries, streams, rivers, wetlands, ponds, and lakes, with setbacks indicated, and including FEMA floodplain data based on most updated FIRM.
- 5. Location, size, shape of all buildings, existing and proposed, with elevation of lowest floor indicated.
- 6. Location and dimensions of existing or proposed on-site sewage systems.
- 7. Location of all propane tanks, fuel tanks or other liquid storage tanks whether above ground or below ground level.
- 8. Location and dimensions of any proposed pipeline placement(s) into floodplain/floodway.
- 9. Location and dimensions of any roadway development into floodplain/floodway. (Includes initial development access roads)
- 10. Location and dimensions of any bridge and/or culvert development into floodplain/floodway.
- 11. Location and dimensions of any storage yard or facility into the floodplain/floodway.
- 12. Location of any existing utilities and/or proposed utility placement and/or displacement.
- 13. Location, dimensions and depth of any existing or proposed fill on site.
- 14. A survey showing the **existing ground elevations** of at least location on the building site. **ELEVATION NOTE**: All vertical datum will reference either NGVD 29 or NAVD 88. Assumed datum will not be acceptable unless the property is located in an area where vertical datum has not been published. For those areas where vertical datum has not been established, a site plan with contours, elevations using assumed datum, high water marks and existing water levels of sloughs, rivers, lakes or streams and proposed lowest floor elevation.

### **Applicant**

Please read print name, sign and date below:

- I certify that I am authorized to submit this application for the primary project developer.
- I certify that the information included in this application is to the best of my knowledge true and complete.
- I certify that all required Federal, State, and local permits required by law and/or ordinance for
  the above described development of this project have been properly attained, are current and
  valid, and must be presented with this application before a Doddridge County Floodplain Permit
  may be issued.
- I understand that if in the course of the development project additional permits become
  required that were not needed during the initial proposal, the primary developer must notify the
  Doddridge County Floodplain Manager within 48 hours of such need, and that a "Stop Work"
  order may be issued for all project work directly impacting the floodplain or floodway, until such
  time the required additional permits are acquired.
- I understand that once the floodplain permit is submitted, the application will be entered into
  official public record at the next regularly scheduled Doddridge County Commission meeting
  after the date of submittal.
- I understand that from the date of submittal of the fully completed permit application, the
  Doddridge County Floodplain Manager has ninety (90) days to make a determination to either
  grant or deny said permit application. During this approval period, the Doddridge County
  Floodplain Manager may, at his or her discretion, conduct a review and/or additional study of
  provided documentation by means of an independent engineering firm. All costs associated with
  said review and/or study must be reimbursed to the County before issuance of approved permit.
- I understand that during the approval period, the Doddridge County Floodplain Manager of designee may at his or her discretion conduct site visits and document conditions of proposed development pursuant to the permit application.
- I understand that once the Floodplain Permit is granted, the permit will be entered into official public record at the next scheduled Doddridge County Commission meeting after the date of issuance. Appeals to the permit may be made no later than twenty (20) days after said issuance. If a valid appeal is submitted, as determined by the Doddridge County Floodplain Manager, a "Stop Work" order will be issued for all project development directly involving the floodplain or floodway. A public hearing by the Doddridge County Appeals Board will be scheduled no less than ten (10) days after the next regularly scheduled Doddridge County Commission meeting.
- I understand that all decisions of the Doddridge County Appeals Board shall be final.
- I understand issuance of a Floodplain Permit authorizes me to proceed with construction as proposed. A Certificate of Compliance is required upon substantial completion of the project.
- In signing this application, the primary developer hereby grants the Doddridge County Floodplain Manager or designee the right to enter onto the above---described location to inspect the development work proposed, in progress, and/or completed.
- I understand that if I do not follow exactly the site---plan submitted and approved by this permit
  that a "Stop Work" order may be issued by the County Floodplain Manager and that I must
  stop all construction immediately until discrepancies of actual work vs. proposed work is
  resolved.

		1	<b>.</b> .	
Applicant Signature:_	15-1-	A Comment	Date:	05-03-18

Applicant Printed Name: Brian C. Sheppard, VP Eastern Pipeline Operations



## United States Department of the Interior



## FISH AND WILDLIFE SERVICE

West Virginia Field Office 90 Vance Drive Elkins, West Virginia 26241

April 26, 2018

Mr. Karl Kratzer Dominion Energy Environmental Services, Inc. 5000 Dominion Boulevard Glen Allen, Virginia 23060

Re: LL Tonkin Compressor Facility - Bridge Replacement, Doddridge County, WV

(FWS File Number 2018-I-0485)

#### Dear Mr. Kratzer:

This letter is in response to your request dated February 21, 2018, and supplemental information dated April 16, 2018, for information regarding the potential occurrence of federally listed endangered and threatened species and their designated critical habitats within the vicinity of the project mentioned above. Dominion Energy Transmission, Inc. (Dominion) proposes to replace an existing bridge that provides access to the LL Tonkin Compressor Facility in Doddridge County, West Virginia. These comments are provided pursuant to the Endangered Species Act (ESA; 87 Stat. 884, as amended; 16 U.S.C. 1531 et seq.). This project has been assigned FWS File Number 2018-I-0485; please reference this number in all future correspondence.

The U.S. Fish and Wildlife Service (Service) has determined that four federally listed species may occur within the project area and may be affected by the construction of this project. These are the endangered Indiana bat (*Myotis sodalis*), snuffbox mussel (*Epioblasma triquetra*), and clubshell mussel (*Pleurobema clava*), and the threatened northern long-eared bat (*Myotis septentrionalis*) (NLEB).

**Project History** 

Upgrades to the LL Tonkin Compressor Facility were included in the Monroe to Cornwell Project (FWS File Number 2014-TA-1030). These upgrades included the construction of a temporary bridge spanning Middle Island Creek. In correspondence dated January 7, 2015, the Service stated that the Monroe to Cornwell Project was not anticipated to adversely affect any federally listed species. Dominion is now proposing to replace the existing permanent bridge to the LL Tonkin Compressor Facility. For a complete project history, please reference Dominion's April 16, 2018, email.

Mr. Karl Kratzer April 26, 2018

#### Federally Listed Bats

The Indiana bat and NLEB may use the project area for foraging and roosting between April 1 and November 15. Indiana bat summer foraging habitats are generally defined as riparian, bottomland, upland forest, and old fields or pastures with scattered trees. Roosting/maternity habitat consists primarily of live or dead hardwood tree species which have exfoliating bark that provides space for bats to roost between the bark and the bole of the tree. Tree cavities, crevices, splits, or hollow portions of tree boles and limbs also provide roost sites. In West Virginia, the U.S. Fish and Wildlife Service (Service) considers all forested habitat containing trees greater than or equal to 5 inches in diameter at breast height to be potentially suitable as summer roosting and foraging habitat for the Indiana bat.

Indiana bats feed on emerged aquatic and terrestrial flying insects. Moths, caddisflies, flies, mosquitoes, and midges are major prey items. Aquatic insects that have concentrated emergences or that form large mating aggregations above or near water appear to be preferred prey items. As a result, streams, wetlands, and associated riparian forests are often preferred foraging habitats for pregnant and lactating Indiana bats. Indiana bats also forage within the canopy of upland forests, over clearings with early successional vegetation (e.g., old fields), along the borders of croplands, along wooded fencerows, and over farm ponds in pastures. Increased erosion and sedimentation of streams reduces diversity and biomass of benthic invertebrates, i.e. insects. Some projects propose impacts to aquatic features such as streams or wetlands, which could result in a decrease in insects available to both bat species for foraging.

Similar to the Indiana bat, NLEB foraging habitat includes forested hillsides and ridges, and small ponds or streams. NLEB are typically associated with large tracts of mature, upland forests with more canopy cover than is preferred by Indiana bats. NLEB seem to be flexible in selecting roosts. They choose roost trees based on suitability to retain bark or provide cavities or crevices, and this species is known to use a wider variety of roost types than the Indiana bat. Males and non-reproductive females may also roost in cooler places like caves and mines. Although rare, this bat has also been found roosting in structures like barns and sheds.

Indiana bats and NLEB use caves or mine portals for winter hibernation between November 15 and March 31. These species also use the hibernacula and the areas around them for fall-swarming and spring-staging activity (August 15 to November 14 and April 1 to May 14, respectively). Some males have been known to stay close to the hibernacula during the summer and may use the hibernacula as summer roosts. There may be other landscape features being used as hibernacula by NLEB during the winter that have yet to be documented.

The Service does not anticipate that this project is likely to adversely affect the Indiana bat because your project: 1) will affect a minimal amount (0.05 acres) of potential Indiana bat foraging or roosting habitat; 2) is not within any of the Indiana bat hibernacula or summer use buffers described above; 3) will not affect any potential caves or mines that could be used as hibernacula for this species; and 4) effects to aquatic features used for foraging habitat will be insignificant.

The NLEB may occur within the range of the proposed project, and may be affected by the proposed construction and operation of this project. Any take of NLEB occurring in conjunction

Mr. Karl Kratzer April 26, 2018

with these activities that complies with the conservation measures (as outlined in the 4(d) rule), as necessary, is exempted from section 9 prohibitions by the 4(d) rule and does not require site specific incidental take authorization. Note that the 4(d) rule does not exempt take that may occur as a result of adverse effects to hibernacula and that no conservation measures are required as part of the 4(d) rule unless the proposed project: 1) involves tree removal within 0.25 miles of known NLEB hibernacula; or 2) cuts or destroys known, occupied maternity roost trees or any other trees within a 150-foot radius around known, occupied maternity tree during the pup season (June 1 to July 31). This proposed project is not located within any of these radii around known hibernacula or roost trees and will not affect any known NLEB hibernacula, therefore any take of NLEB associated with this project is exempted under the 4(d) rule and no conservation measures are required.

Federally Listed Mussels

The proposed project occurs within the vicinity of Middle Island Creek, which provides habitat for clubshell and snuffbox mussels. Freshwater mussels feed by filtering food particles from the water column. Juvenile and adult freshwater mussels have been documented to feed on detritus, diatoms, phytoplankton, and zooplankton. Freshwater mussels rely on fish to complete their lifecycles. When mussel larvae (glochidia) are released into the water by adult females, they must attach themselves within a few days to an appropriate fish host, which they then parasitize for a short time while developing into juvenile mussels. The loss of many historic populations and the continuing degradation of current populations are due to the impacts of impoundments, navigation projects, water quality degradation from agricultural and industrial wastes, and deforestation and other forms of habitat alteration, including gravel and sand dredging. Impacts that directly affect the species also include reduction or elimination of fish hosts.

Freshwater mussels play a key role in the health of aquatic environments by helping to maintain water quality, cycle nutrients, stabilize substrates, remove suspended sediments, and create habitat complexity (Anderson and Kreeger 2012). Mussels remove suspended particulates from water by filter-feeding, which involves straining water over gills to remove microscopic food particles. Filter-feeding makes mussels very sensitive to excessive sedimentation caused by runoff from streambank and upland activities. Excessive sedimentation into a stream reduces suitable habitat for mussels and can smother them, causing death. Excessive suspended sediments can also impair feeding processes of mussels, leading to acute short-term or chronic long-term stress. Both excessive sedimentation and excessive suspended sediments can lead to reduced populations of mussels and therefore reduced benefits to the ecosystem (Anderson and Kreeger 2010; Box and Mossa 1999; Ellis 1931; Ellis 1936; Houp 1993).

Due to the proximity of the project to Middle Island Creek, the project could cause increased sedimentation that could adversely affect mussels. Dominion proposes to replace the existing bridge on existing piers and no work will occur below the ordinary high water mark. In correspondence dated February 21, 2018, and April 16, 2018, Dominion stated their commitment to erosion and sedimentation best management practices to include triple stacked compost filter sock and routine inspection of best management practices. These best management practices will help minimize adverse effects of sedimentation and erosion to waterways that provide habitat to federally listed mussels near the action area. In addition, Dominion will comply with the requirements set forth in the 2012 Erosion and Sediment Control Field Oil and Gas Manual.

Mr. Karl Kratzer April 26, 2018

Based on Dominion's commitment to avoid instream work and to implement erosion and sedimentation control best management practices, the Service does not anticipate that this project is likely to adversely affect any federally listed mussel species.

### Summary

Based on the information provided to us, the Service has concluded that no federally listed species are expected to be adversely affected by the project and any that any take of NLEB associated with this project is exempt under the 4(d) rule.

This letter provides technical assistance only and does not serve as a completed section 7 consultation document. If there is a Federal nexus for the project (e.g., Federal funding provided, Federal permits required to construct), no tree clearing or any project construction activities on any portion of the parcel should occur until consultation under section 7 of the ESA, between the Service and the Federal action agency, is completed. Section 7 consultation is not complete until the federal action agency submits a determination of effects to this office, the Service concurs with the federal action agency's determination, and the federal action agency agrees to incorporate all the proposed conservation measures as mandatory conditions for any permit decision rendered for this project. All measures must be installed prior to the start of any construction, and all measures must be implemented as proposed and be maintained until construction is complete. If there is no Federal nexus associated with this project, then no further coordination with this office is required.

Should project plans change or amendments be proposed that we have not considered in your proposed action, or if additional information on listed and proposed species becomes available, or if new species become listed or critical habitat is designated, this determination may be reconsidered. This technical assistance letter regarding effects to any federally listed endangered and threatened species does not apply if erosion and sedimentation measures fail due to improper maintenance. If erosion and sedimentation measures fail, further consultation with the Service will be required.

If you have any questions regarding this letter, please contact Amanda Selnick of my staff at (304) 636-6586, Ext. 24, or amanda selnick@fws.gov, or at the letterhead address.

Sincerely,

John Schmidt Field Supervisor

#### Literature Cited

- Anderson, R. M. and D. A. Kreeger. 2010. Potential for impairment of freshwater mussel populations in DRBC special protection waters as a consequence of natural gas exploratory well development. Unpublished report prepared for U.S. Fish and Wildlife Service and the Partnership for the Delaware Estuary. State College, Pennsylvania.
- Box, J. M. and J. Mossa. 1999. Sediment, land use, and freshwater mussels: prospects and problems. *Journal of the North American Benthological Society*. 18:99-117.
- Ellis, M. M. 1931. Some factors affecting the replacement of the commercial fresh-water mussels. U.S. Department of Commerce Bureau of Fisheries. Fishery Circular. 7:1-10.
- Ellis, M. M. 1936. Erosion silt as a factor in aquatic environments. Ecology. 17:29-42.
- Houp, R. E. 1993. Observations on long-term effects of sedimentation on freshwater mussels (Molusca: Unionidae) in the North Fork of Red River, Kentucky. Transactions of the Kentucky Academy of Science. 54:93-97.

# APPENDIX B PHOTOS





PHOTO 1
Existing compressor station.

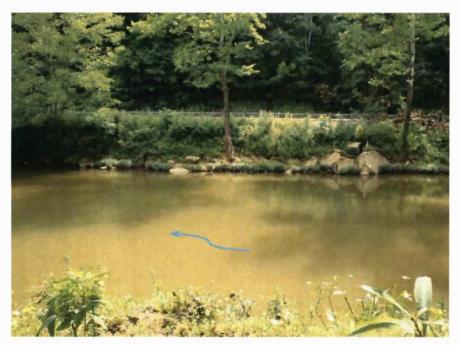


PHOTO 2
Typical stream section.





PHOTO 3 Existing bridge (downstream side).



# APPENDIX C HYDROLOGY CALCULATIONS



## EXCERT FROM DODDRIDGE COUNTY FIS

## 3.1 Hydrologic Analyses

Hydrologic analyses were carried out to establish the peak discharge-frequency relationships for each flooding source studied in detail affecting the county.

Discharge-frequency curves were developed on a regional basis that applies to West Virginia (References 3 and 4). For the streams studied by detailed methods, 1-percent-annual-chance flood elevations were determined through discharge-frequency relations and the Manning equation. Within the Town of West Union, flood elevations were determined through streamflow-station data relationships and the Manning's equation.

Peak discharge-drainage area relationships for each stream studied by detailed methods are presented in Table 2, "Summary of Discharges".

Table 2 - Summary of Discharges

FLOODING SOURCE AND LOCATION	DRAINAGE AREA (SQ. MILES)	PEAK DISCHARGE (CFS) 1-PERCENT- <u>ANNUAL- CHANCE</u>
MIDDLE ISLAND CREEK		
Upstream of Doddridge-Tyler County boundary	134.78	15,200
Approximately 0.1 mile downstream of		
confluence of Piggin Run	120.06	13,080
BUCKEYE CREEK		
At confluence with Middle Island Creek	38.62	7,350
Downstream of confluence of Long Run	22.62	5,150
Upstream of confluence of Greenbrier Creek	9.41	3,050
Downstream of confluence of Traugh Fork	1.52	1,310
MEATHOUSE FORK		
At confluence with Middle Island Creek	66.84	9,600
Downstream of confluence of Toms Fork	50.47	8,200
Downstream of confluence of Brushy Fork	29.87	6,050
Downstream of confluence of Laurel Run and		
Big Isaac Creek	3.76	2,230
MCELROY CREEK		
Upstream of confluence of Flint Run	61.95	9,250
Upstream of confluence of Rigging Run	51.23	8,300
Downstream of confluence of Talkington Fork	39.18	7,100
Downstream of confluence of Robinson Fork and		
Big Battle Run	20.75	4,900

# APPENDIX D HYDRAULIC CALCULATIONS



## EXCERT FROM DODDRIDGE COUNTY FIS

using the USACE HEC-2 step-backwater computer program, and the results were published in a special flood hazard information report (References 5 and 6). Flood profiles were drawn showing computed water-surface elevations for floods of the selected recurrence intervals.

Channel roughness factors (Manning's "n") used in the hydraulic computations were assigned on the basis of field surveys of the stream and floodplain areas. For Middle Island Creek, channel "n" values range from 0.040 to 0.045 and overbank "n" values range from 0.050 to 0.070. For Buckeye Creek and Meathouse Fork, channel "n" values range from 0.055 to 0.080.

The hydraulic analyses for this study were based on unobstructed flow. The flood elevations shown on the profiles are thus considered valid only if hydraulic structures remain unobstructed, operate properly, and do not fail.

Qualifying benchmarks within a given jurisdiction that are catalogued by the National Geodetic Survey (NGS) and entered into the National Spatial Reference System (NSRS) as First or Second Order Vertical and have a vertical stability classification of A, B or C are shown and labeled on the FIRM with their 6-character NSRS Permanent Identifier.

Benchmarks catalogued by the NGS and entered into the NSRS vary widely in vertical stability classification. NSRS vertical stability classifications are as follows:

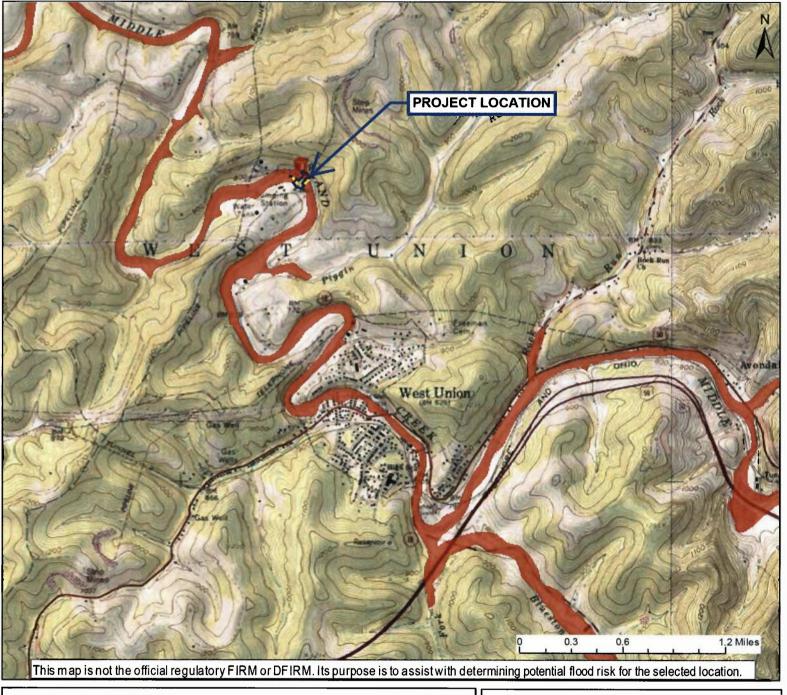
- Stability A: Monuments of the most reliable nature, expected to hold position/elevation (e.g. mounted in bedrock)
- Stability B: Monuments which generally hold their position/elevation (e.g. concrete bridge abutment)
- Stability C: Monuments which may be affected by surface ground movements (e.g. concrete monument below frost line)
- Stability D: Mark of questionable or unknown vertical stability (e.g. concrete monument above frost line, or steel witness post)

In addition to NSRS benchmarks, the FIRM may also show vertical control monuments established by a local jurisdiction; these monuments will be shown on the FIRM with the appropriate designations. Local monuments will only be placed on the FIRM if the community has requested that they be included, and if the monuments meet the aforementioned NSRS inclusion criteria.

To obtain current elevation, description, and/or location information for benchmarks shown on the FIRM for this jurisdiction, please contact the Information Services Branch of the NGS at (301) 713-3242, or visit their Web site at <a href="https://www.ngs.noaa.gov">www.ngs.noaa.gov</a>.

It is important to note that temporary vertical monuments are often established during the preparation of a flood hazard analysis for the purpose of establishing local vertical control. Although these monuments are not shown on the FIRM, they may be found in the Technical Support Data Notebook associated with the FIS report and FIRM for this community. Interested individuals may contact FEMA to access these data.

## WV Flood Map



#### User Notes:

Dominion Energy Transmission, Inc. LL Tonkin Permanent Bridge



Flood Hazard Zone



Flood Point of Interest

### Disclaimer:

The online map is for use in administering the National Flood Insurance Program. It does not necessarily identify all areas subject to flooding, particularly from local drainage sources of small size. To obtain more detailed information in areas where Base Flood Elevations have been determined, users are encouraged to consult the latest Flood Profile data contained in the official flood insurance study. These studies are available online at www.msc.fema.gov. WV Flood Tool (http://www.MapWV.gov/flood) is supported by FEMA, WV NFIP Office, and WV GIS Technical Center.

Map created on December 12, 2017

#### Flood Hazard Area:

Flood Hazard Area: Location is WITHIN the FEMA 100-year floodplain.

FEMA Issued Flood Map: 54017C0120C

Watershed (HUC8): Little Musringum-Middle Island

**Elevation:** About 753 ft

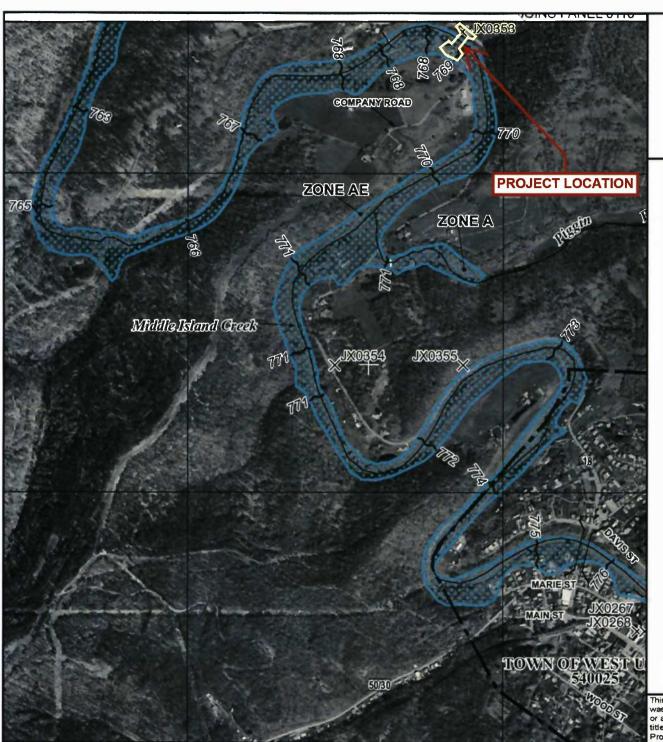
**Location (long, lat):** (80.781238 W,39.311925 N)

Location (UTM 17N): (518860, 4351414)

Contacts: Doddridge

CRS Information: N/A

Parcel Number: No Parcel





MAP SCALE 1" = 1000'

500 0 1000 2000 FEET



PANEL 0120C

## **FIRM**

FLOOD INSURANCE RATE MAP DODDRIDGE COUNTY, WEST VIRGINIA AND INCORPORATED AREAS

**PANEL 120 OF 325** 

(SEE MAP INDEX FOR FIRM PANEL LAYOUT)

CONTAINS

 COMMUNITY
 NUMBER
 PANEL
 SUFFIX

 DODDRIDGE COUNTY
 540024
 0120
 C

 WEST UNION, TOWN OF
 540025
 0120
 C

Notice to User: The Map Number shown below should be used when placing map orders; the Community Number shown above should be used on insurance applications for the subject community.



MAP NUMBER 54017C0120C MAP REVISED OCTOBER 4, 2011

Federal Emergency Management Agency

This is an official copy of a portion of the above referenced flood map. It was extracted using F-MIT On-Line. This map does not reflect changes or amendments which may have been made subsequent to the date on the title block. For the latest product information about National Flood Insurance Program flood maps check the FEMA Flood Map Store at www.msc.fema.gov

## HYDROLOGY AND HYDRAULICS REPORT FOR FLOODPLAIN IMPACT

**FOR** 

# Dominion Transmission, Inc.

# MONROE TO CORNWELL PROJECT - LL TONKIN STATION

WEST UNION
DODDRIDGE COUNTY, WEST VIRGINIA

**July 2015** 

Submitted for:
Dominion Transmission, Inc.
445 West Main St.
Clarksburg, West Virginia 26301

Prepared By:



FOSTER PLAZA 6 681 ANDERSEN DRIVE, SUITE 400 PITTSBURGH, PA 15220

Job # 60327471

## **HYDROLOGY & HYDRAULICS REPORT**

## **TABLE OF CONTENTS**

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1.0	PROJECT OVERVIEW	1
2.0	TECHNICAL APPROACH	1
3.0	ANALYSIS RESULTS	3

## **APPENDICES**

## Appendix A. Figures

Figure 1 - Location Map

Figure 2 – FEMA Firmette

Figure 3 – Hydrology & Hydraulics Plan

Appendix B. Photos

Appendix C. Hydrology Calculations

Appendix D. Hydraulics Calculations



# HYDROLOGY & HYDRAULICS REPORT FOR LL TONKIN STATION

# 1.0 PROJECT OVERVIEW

Dominion Transmission, Inc. has proposed expansion of an existing natural gas compressor station in West Union, Doddridge County. Upon completion of construction activities, proposed compressor station facilities and grading will remain.

The proposed project is located in the Middle Island Creek watershed. The proposed grading for the compressor station expansion is located within the FEMA Flood Zone AE (with no FEMA-determined floodways) along Middle Island Creek.

# 1.1 Existing Conditions

The stream and floodplain were surveyed by others during a site investigation. A site investigation of the project area identified the bottom width of the existing stream at the project location to be approximately seventy feet (70') and the overall depth of the channel to be approximately thirteen feet (13'). Channel side slopes range from approximately 1.5H:1V to steeper than 1H:1V and the channel top width is approximately ten feet (80'). The flow depth was observed to be five and a half feet (5.5'). There is an existing road crossing approximately 830 feet downstream of the proposed grading.

The existing road crossing consists of a steel bridge with concrete piers and abutments. The existing crossing is proposed to remain in place. The drainage area for this project is approximately 123 square miles. See **Appendix A** for figures.

# 1.2 Proposed Conditions

Proposed grading activities will take place to expand the existing Tonkin compressor station and will involve fill material being placed within the 100-year floodplain. See **Appendix A** for a figure of the site.

# 2.0 TECHNICAL APPROACH

The proposed facilities were analyzed for their impacts to the existing stream. A hydrology and hydraulics analysis was completed for the existing conditions and the proposed conditions. The effective FEMA Flood Insurance Study (FIS) for this project is "Doddridge County West Virginia and Incorporated Areas," effective October 4, 2011; this study was created as a digital conversion of the initial Countywide FIS. Since the project is located in a FEMA detailed study zone AE, data from the FIS was used in the hydraulic and hydrologic analyses.

# 2.1 Precipitation Data

The peak discharge calculated in the FEMA Flood Insurance Study (FIS) was used; therefore, no precipitation data was obtained.



# 2.2 Hydrology Analysis

The results of the hydrologic analyses in the FIS show a drainage area of 134.78 square miles, with a peak discharge 1-percent-annual-chance of 15,200 cfs on the nearest downstream cross section on Middle Island Creek. This discharge from the FIS was used as the 100-year design flow, providing a conservative design flow. A summary of the runoff results is contained in Table 1 below.

TABLE 1.

Summary of D	esign Flows (cfs)
	100-Year Design Flow
Middle Island Creek	15, 200

The detailed input parameters and runoff results are contained in the hydrology calculations included in **Appendix C**.

# 2.3 Hydraulic Analysis

Hydraulic calculations were performed to analyze the anticipated impacts of constructing the proposed facilities. This analysis was completed to determine whether the proposed activity complied with Section 4.3 of the Doddridge County Floodplain Ordinance.

HEC-RAS Version 4.1.0 was used for the hydraulic calculations. Both an existing and proposed geometry were created. The data for the cross-sections and existing bridge came from surveyed points (vertical datum NAVD 88). Points were taken across the floodplain and surrounding area, and nine cross sections were taken along the stream. Three-dimensional surfaces were created in AutoCAD Civil 3D from this data and additional cross sections were cut from this surface. The proposed geometry uses blocked obstructions in the locations of proposed fill.

The Manning's "n" values used for Middle Island Creek in the FIS ranged from 0.040 to 0.045 in the channel and 0.050 to 0.070 in the overbank. Our analysis used a value of 0.040 in the channel, corresponding to a clean, winding channel with some pools and shoals. Values in the overbank ranged between 0.050 and 0.070, depending on the amount of vegetation and debris at a particular location.

Expansion and contraction coefficients of 0.3 and 0.1, respectively, were used at all cross sections, except the two bounding cross sections of the existing bridge (River Stations 826.02 and 769.3), where values of 0.5 and 0.3 were used.

The bridge modeling approach was Energy (Standard Step) for the Low Flow method and Energy Only (Standard Step) for the High Flow method, though the bridge is not submerged in the 100-year design storm.

Both the existing and proposed condition use the same steady flow data and are run using a subcritical flow regime. The boundary condition is a known downstream water surface elevation. The water surface elevation 767.8 feet, was obtained from the water surface profile in plate 22P of the FIS, which also uses the vertical datum NAVD 88. The discharge used in the model is described in Section 2.2 of this report, and shown in Table 1.



TABLE 2.

Summai	y of Water Su	rface Elevati	ons
River Station	Existing 100-year	Proposed 100-year	Difference in WSE
2304.58	771.16	771.20	0.04
1970.3	770.82	770.86	0.04
1900	770.72	770.78	0.06
1800	770.08	770.13	0.05
1700	769.80	769.80	0.00
1636.04	769.76	769.77	0.01
1600	769.78	769.79	0.01
1500	769.72	769.72	0.00
1422.55	769.68	769.68	0.00
916.96	768.89	768.89	0.00
868.08	768.86	768.86	0.00
826.02	768.86	768.86	0.00
769.3	768.60	768.60	0.00
721.29	768.60	768.60	0.00
289.25	767.80	767.80	0.00

The design input parameters and analysis results are contained in the hydraulic calculations included in **Appendix D**.

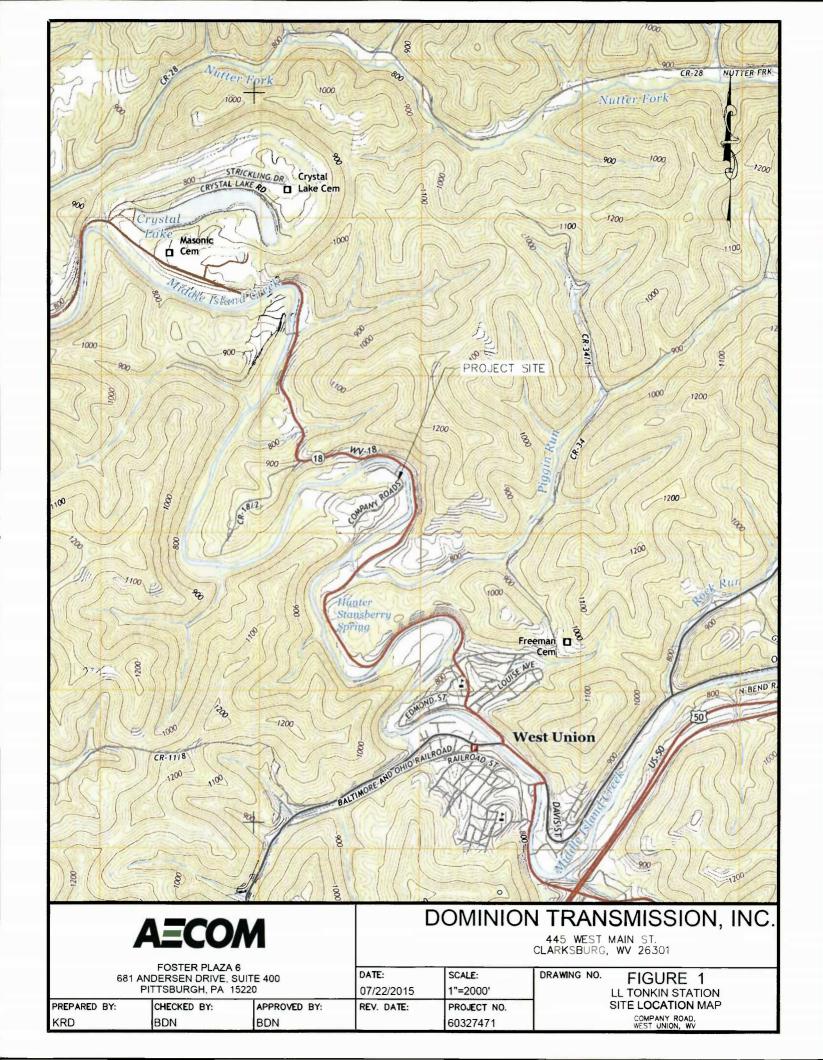
# 3.0 ANALYSIS RESULTS

A comparison of the existing and proposed conditions shows that the proposed structure will have virtually no impact on the flow characteristics of the stream that were analyzed or on adjacent properties. These calculations also show that proposed construction does not increase tailwater velocity. Further analysis shows that the existing bridge structure is able to safely convey the 100-year design flow without overtopping the existing road crossing. Refer to **Appendix D** for detailed results of the analysis.



# APPENDIX A FIGURES







May 3, 2018

# **BY CERTIFIED MAIL NO.:**

Mr. Greg Eidel, Doddridge County Floodplain Manager Doddridge County Commission 118 East Court Street, Room 102 West Union, West Virginia 26456

RE: <u>Dominion Energy Transmission, Inc.</u> LL Tonkin Compressor Station Bridge

No Rise Certificate

**Doddridge County, West Virginia** 

Dear Mr. Eidel:

Dominion Energy Transmission, Inc. (Dominion) is pleased to submit the attached No-Rise Certificate for the LL Tonkin Compressor Station Bridge. The proposed work includes replacing the superstructure on the existing permanent bridge over Middle Island Creek and removing the temporary bridge installed as part of the compressor station expansion project. The project is located in the Middle Island Creek Zone AE Flood Hazard area, according to the Flood Insurance Rate Map (FIRM) for Doddridge County, map panel 54017C0120C with a map revised date of October 4, 2011.

Included with this application is a letter from Dominion's consultant, Civil & Environmental Consultants, stamped by a professional engineer, attesting to the no adverse effects of the reconstructed superstructure.

Should you have any questions or require further information, please do not hesitate to contact Karl R. Kratzer at (804) 273-2914 or <u>Karl.R.Kratzer@dominionenergy.com</u> or Mr. Joshua Cook at (681) 842-3908 or <u>Joshua.D.Cook@dominionenergy.com</u>.

Sincerely

Richard B. Gangle

**Director Environmental Services** 

cc: Karl Kratzer

LL Tonkin Bridge Replacement Doddridge County Floodplain May 3, 2018

Bcc: Amanda Tornabene

Richard B. Gangle Neil Robinson

Josh Cook

Greg Linder, CEC



# George Eidel <doddridgecountyfpm@gmail.com>

# Fwd: Authorization Activity - LL Tonkin Bridge (UNCLASSIFIED)

1 message

**Robert C. Orndorff** <robert.c.orndorff@dominionenergy.com>
To: "doddridgecountyfpm@gmail.com" <doddridgecountyfpm@gmail.com>

Fri, May 4, 2018 at 9:27 AM

Bob Orndorff Director of State and Local Government Affairs Dominion Energy Services, Inc 304 641-3608

Begin forwarded message:

From: "Karl R Kratzer (Services - 6)" <Karl.R.Kratzer@dominionenergy.com>

Date: May 4, 2018 at 9:23:03 AM EDT

To: "Kester, Jacqulyn" < jkester@cecinc.com>

Cc: "Robert C. Orndorff (Services - 6)" <robert.c.orndorff@dominionenergy.com>, "Joshua D Cook

(GasInfrastructure - 2)" <Joshua.D.Cook@dominionenergy.com>

Subject: FW: Authorization Activity - LL Tonkin Bridge (UNCLASSIFIED)

Jacqulyn:

Nicely done. Please forward this to George Eidel at your earliest convienience.

Thank you.

Karl R. Kratzer
Dominion Energy Environmental Services
5000 Dominion Boulevard
Glen Allen, Virginia 23060
(804) 273-2914 - Desk
(804) 356-9282 - Cell
karl.r.kratzer@dominionenergy.com

----Original Message-----

From: Fannin, Adam E CIV USARMY CELRH (US) [mailto:Adam.E.Fannin@usace.army.mil]

Sent: Friday, May 04, 2018 9:20 AM

To: Kester, Jacqulyn

Cc: Karl R Kratzer (Services - 6); ROBINETTE, Lee A CIV USARMY CELRH (US) Subject: [External] RE: Authorization Activity - LL Tonkin Bridge (UNCLASSIFIED)

**CLASSIFICATION: UNCLASSIFIED** 

Good Morning Jacquiyn.

The Corps' authority to regulate waters of the United States is based on the definitions and limits of jurisdiction at 33 CFR 328 and 33 CFR 329. A Section 404 permit must be obtained prior to discharging dredged or fill material into waters of the United States, including wetlands. Section 10 of the Rivers and Harbors Act requires a permit be obtained in advance of any work in, on, over or under a navigable water of the United States.

Middle Island Creek is not a Section 10 waterway with the exception of the Section 10 slackwater extent

from the Ohio River which includes the lower 12.0 miles of Middle Island Creek. Your project is not located in the Section 10 portion of Middle Island Creek, therefore, no Section 10 permit would be required to conduct work in, over, or under the waterway.

Middle Island Creek would be subject to Section 404 of the Clean Water Act. However, based on the description of the proposed work and the plans provided, it does not appear that the proposal would involve the discharge of dredged and/or fill material into Middle Island Creek. Therefore, based on the information provided, no Section 404 permit would be required.

I hope that's helpful. If you need additional information feel free to contact me again.

Sincerely,

Adam Fannin
Regulatory Project Manager
Energy Resource Branch
USACE, Huntington District, CELRH-RD-E
502 8th Street
Huntington, WV 25701
304-399-6901
304-399-5085 (fax)
adam.e.fannin@usace.army.mil

----Original Message-----

From: Kester, Jacqulyn [mailto:jkester@cecinc.com]

Sent: Thursday, May 3, 2018 4:15 PM

To: Fannin, Adam E CIV USARMY CELRH (US) <Adam.E.Fannin@usace.army.mil>

Cc: Karl R Kratzer (Karl.R.Kratzer@dominionenergy.com) < Karl.R.Kratzer@dominionenergy.com>

Subject: [Non-DoD Source] Authorization Activity - LL Tonkin Bridge

Adam,

Dominion Energy, Inc. (Dominion) is proposing to install a permanent bridge deck along CNG Road, which is located off State Route 18 in northwestern Doddridge County, West Virginia. The proposed project entails the replacement of the existing bridge deck with a new 145-foot long by 24-foot wide bridge deck with paved approaches capable of supporting heavy vehicle traffic to allow Dominion with permanent access to their existing LL Tonkin Compressor Station. The proposed activities are located outside of the limits of the compressor facility and the bridge is privately owned. Construction of the new bridge deck will utilize the existing piers located in Middle Island Creek and as such no instream work or activities below the plane of the OHWM will be required. The existing pier caps will be reworked to allow the proposed beams to be placed across them. Additionally, Middle Island Creek is not listed as a traditionally navigable water.

Under the above described scope or work, would this be an activity that would require authorization from your office? I have attached the plans for your reference

Thank you in advance for your time.

Jacqulyn D. Kester / Senior Project Manager

Civil & Environmental Consultants, Inc.

600 Marketplace Ave \* Suite 200 \* Bridgeport, WV 26330

Toll-Free: (855) 488-9539 \* Direct: (304) 848-7121 \* Fax: (304) 933-3327

Mobile: (304) 838-8600 \* Blockedhttp://www.cecinc.com

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**CLASSIFICATION: UNCLASSIFIED** 

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# The Doddridge Independent **PUBLISHER'S CERTIFICATE**

I, Michael D. Zorn, Publisher of The Doddridge Independent, A newspaper of general circulation published in the town of West Union, Doddridge County, West Virginia, do hereby certify that:

Please take notice that on the 4th day of May, 2018, Dominion Energy filed an application for a Floodplain Permit (#18-511) to develop land located at or about Tonkin Station Rd and Rt 18 N Coordinates 39.311925 N, -80.781238 W. The Application is on file with the Doddridge County Floodplain Manager's Office and may be inspected or copied during regular business hours. Any interested persons who desire to comment shall present the same in writing by June 4, 2018 (20 calendar days after the announcement at the regularly scheduled Doddridge County

was published in The Doddridge Independent 2 times commencing on Friday, May 11, 2018 and Ending on Friday, May 18, 2018 at the request of:

# George Eidel, OEM Director Doddridge County Flood Plain

Given under my hand this Friday, May 18, 2018

The publisher's fee for said publication is:

\$ 24.84 1st Run/\$ 18.63 Subsequent Runs This Legal Ad Total: \$43.47

e Doddridge Independent

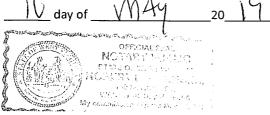
Michael D. Zor

Subscribed to and sworn to before me on

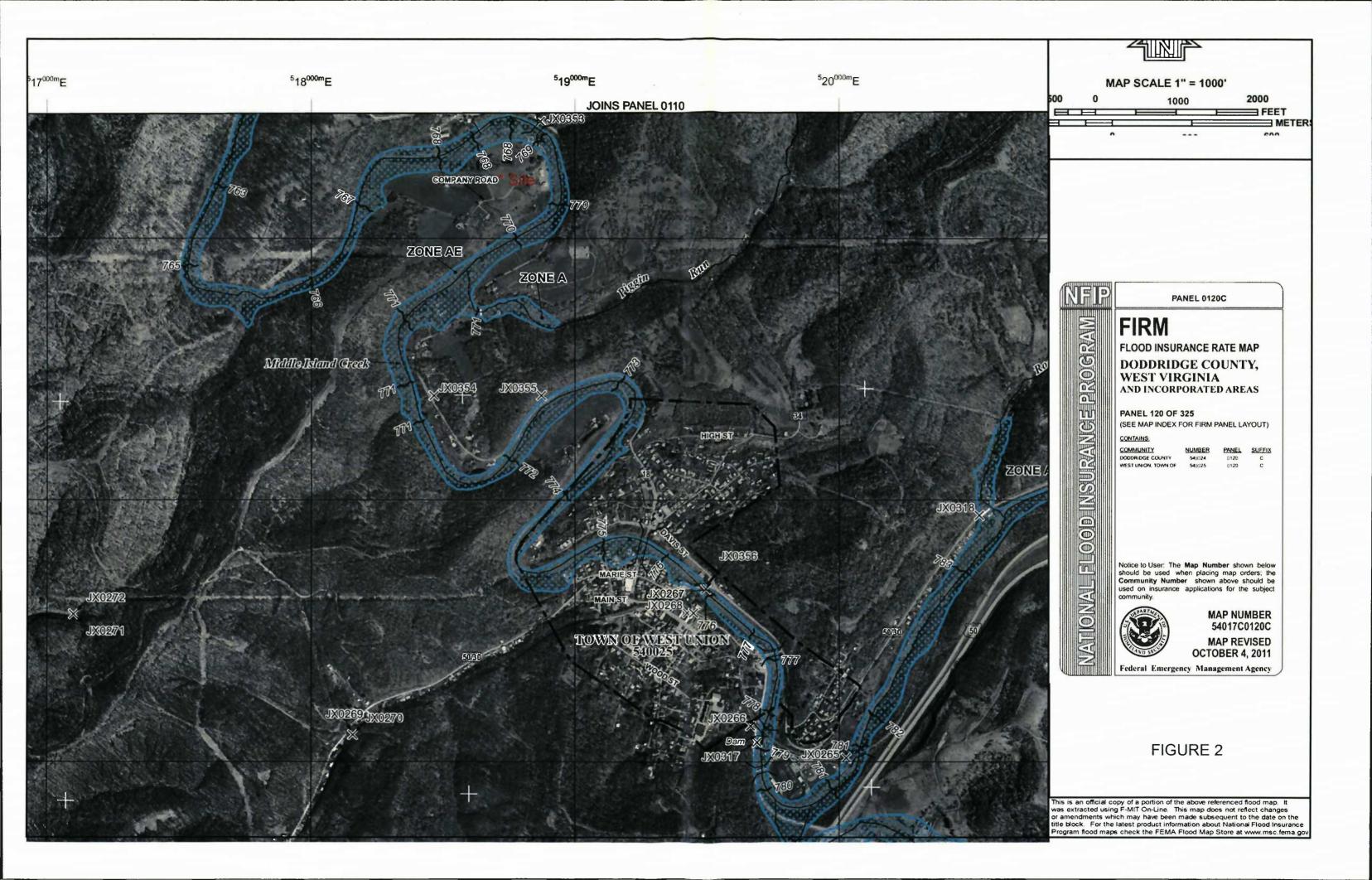
this date:

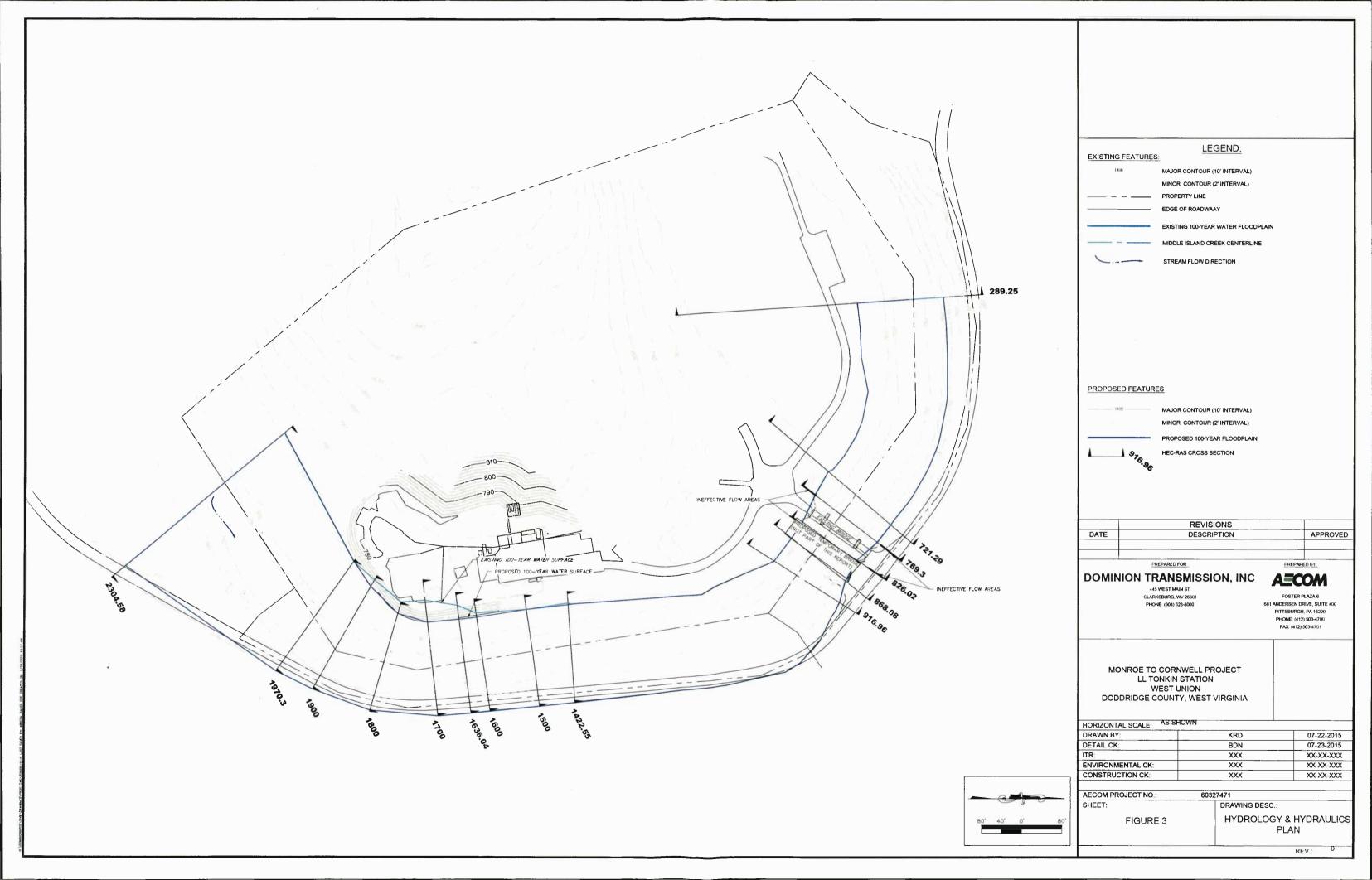
Notary Public in and for Doddridge County

My Commission expires on



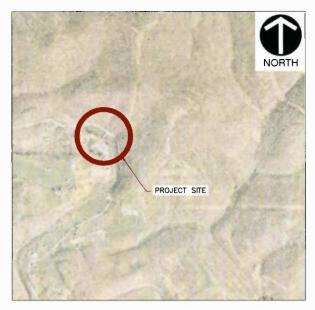
Please take notice that on the 4th day of May, 2018, Dominion Energy filed an application for a Floodplain Permit (#18-511) to develop land located at or about Tonkin Station Rd and Rt 18 N Coordinates 39.311925 N, -80.781238 W. The Application is on file with the Doddridge County Floodplain Manager's Office and may be inspected or copied during regular business hours. Any interested persons who desire to comment shall present the same in writing by June 4, 2018 (20 calendar days after the announcement at the regularly scheduled Doddridge County Commission Meeting) delivered to the Clerk of the County Court at 108 Court Street Ste. 1, West Union, WV 26456. This project is a bridge replacement C2 05/11 - 05/17







# DOMINION ENERGY, INC. LL TONKIN COMPRESSOR STATION BRIDGE DODDRIDGE COUNTY, WEST VIRGINIA



LAYOUT
ALE: 1 INCH = 1,000 FEET

APPROXIMATE CENTER OF PAD: N 039'18'43.32", W 080'46'52.35"

Sheet Number	Sheet Title
1	TITLE SHEET
2	ROADWAY CONSTRUCTION PLAN
3	SITE PLAN
4	SUMMARY OF QUANTITIES
5	GENERAL NOTES
6	STAKE OUT PLAN
7	ABUTMENT 1 PLAN & ELEVATION
8	ABUTMENT 1 FOOTING DETAILS
9	ABUTMENT 1 DETAILS
10	ABUTMENT 2 PLAN & ELEVATION
11	ABUTMENT 2 FOOTING DETAILS
12	ABUTMENT 2 DETAILS
13	PIER 1 DETAILS
14	PIER 2 DETAILS
15	FRAMING PLAN
16	GIRDER ELEVATIONS
17	STEEL DETAILS
18	END DIAPHRAGM DETAILS
19	END DIAPHRAGM DETAILS
20	INTERMEDIATE DIAPHRAGM DETAILS
21	CAMBER ORDINATES
22	BEARING DETAILS
23	DECK PLAN
24	DECK DETAILS
25	DECK DETAILS
26	NEOPRENE STRIP SEAL DAM DETAILS
27	TOP OF DECK ELEVATIONS
28	REINFORCING STEEL LIST
29	REINFORCING STEEL LIST
30	EROSION AND SEDIMENT CONTROL PLA

DISTURBED ACREAGE
0.95 ACRES
TREE CLEARING = 0.05 ACRES

FLOOD PANEL INFORMATION

BY GRAPHICAL PLOTTING ONLY, THE IMPROVEMENT IS LOCATED IN A FEMA DESIGNATED SPECIAL FLOOD HAZARD AREA, ACCORDING TO THE FLOOD INSURANCE RATE MAPS FOR DODDRIDGE COUNTY, MAP # 54017C0120C WITH AN EFFECTIVE DATE OF 10/04/2011.

PERMITS

WEST VIRGINIA GENERAL WATER POLLUTION CONTROL PERMIT FOR STORMWATER ASSOCIATED WITH OIL AND GAS CONSTRUCTION ACTIVITY (NOT REQUIRED)

UNITED STATES ARMY CORPS OF ENGINEERS NATIONWIDE PERMIT 14 (NOT REQUIRED)

OFFICE OF LAND AND STREAMS STREAM ACTIVITY APPLICATION
(NOT REQUIRED)

TRACTOR SHALL CONTACT
UTILITY OF WEST VIRGINIA
AT 800-245-4848
AT LEAST 43 HOURS
OR TO BEGINNING WORK.



EFERENCES

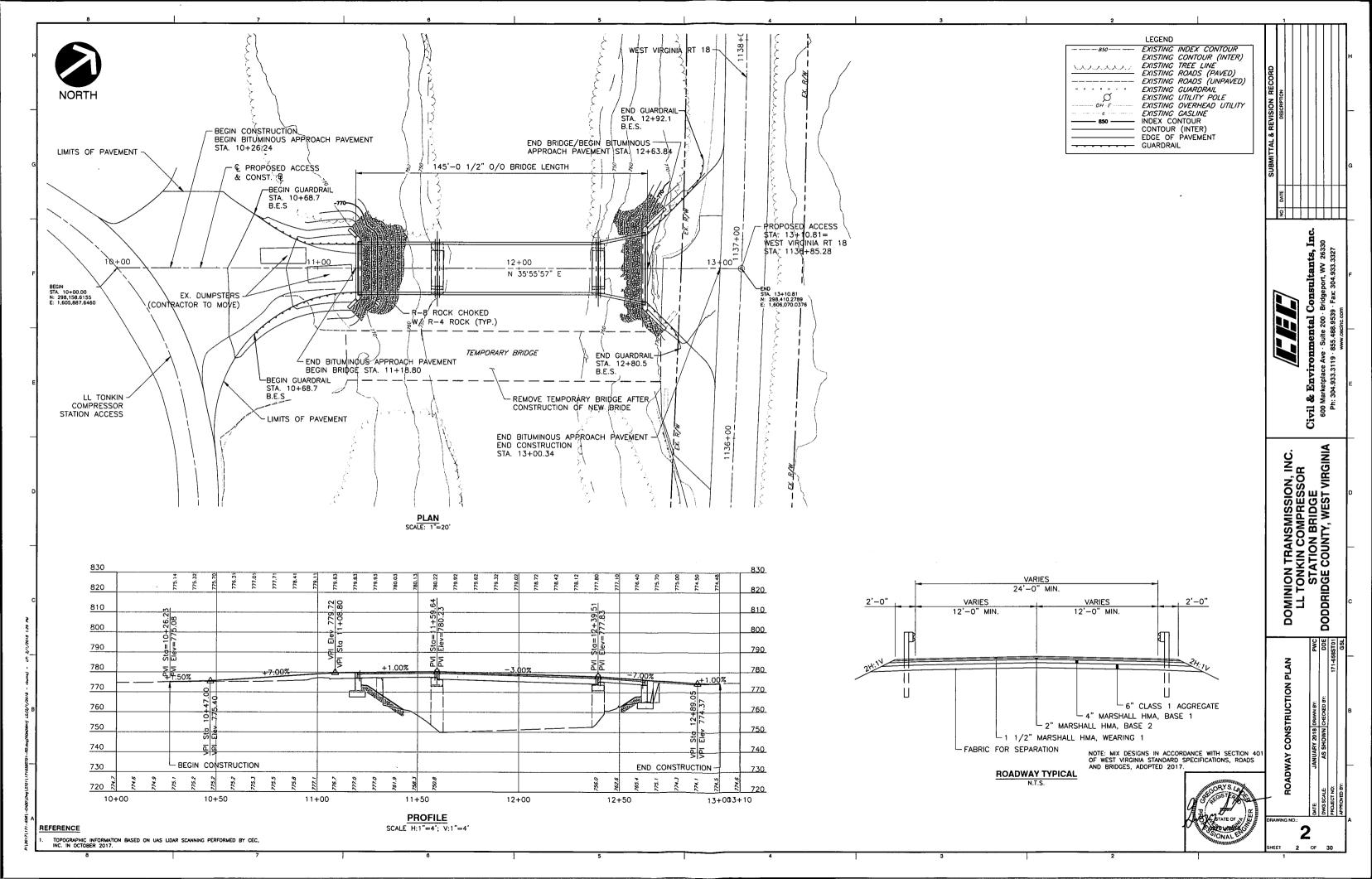
TOPOGRAPHIC INFORMATION BASED ON UAS LIDAR SCANNING PERFORMED BY CEC INC. IN OCTOBER 2017. ISSUED FOR CONSTRUCTION

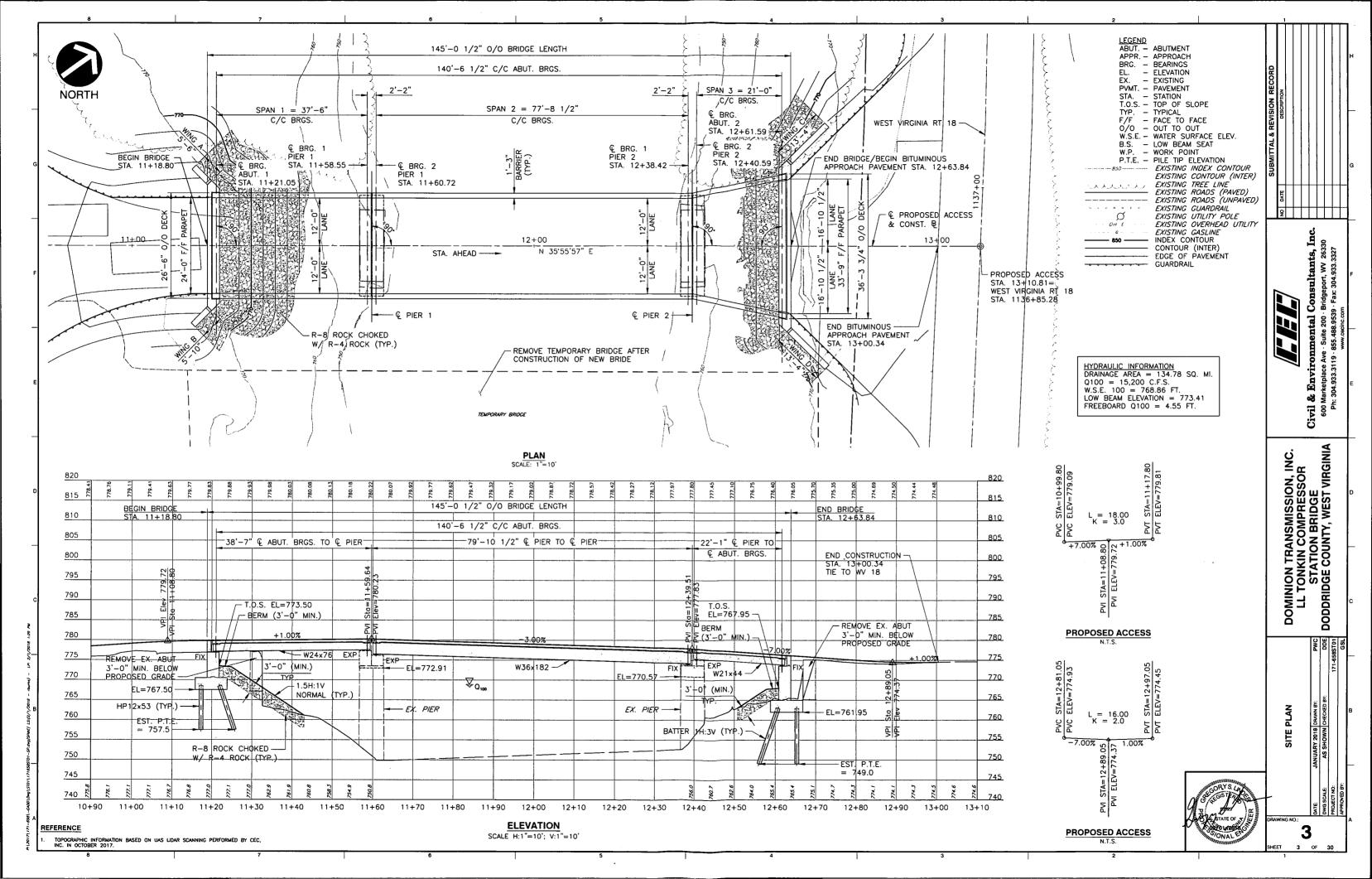
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DOMINION TRANSMISSION, INC. LL TONKIN COMPRESSOR STATION BRIDGE DODDRIDGE COUNTY, WEST VIRGINIA

OF 3





SUMMARY OF		1		
ESTIMATED QUANTITIES				
ITEM NO.	ITEM DESCRIPTION	UNITS	ESTIMATED	AS-BUILT
ROADWAY			QUANTITY	QUANTITY
ROADWAT				
			THE PLANE.	
204001-000	MOBILIZATION	LS	1	
207001-001	UNCLASSIFIED EXCAVATION	CY	350	
207034-000	FABRIC FOR SEPARATION	SY	880	
401001-020	MARSHALL BASE CRSE, SG, TY I	TN	200	
401001-023	MARSHALL BASE CRSE, SG, TY II	TN	100	
401002-020	MARSHALL WEAR CRSE, SG, TY I	TN	75	
307001-000	AGGREGATE BASE COURSE, CLASS I	TN	270	
607001-001	TYPE 1 GUARDRAIL, CLASS I	LF	150	
607006-001	THRIE BEAM GUARDRAIL BRIDGE TRANSITION	EA	4	
607037-001	BUFFER END SECTION	EA	4	
642	TRIPLE STACKED FILTER SOCK	LF	250	
BRIDGE				
203001-000	DISMANTLING STRUCTURE	LS	1	
615001-001	STEEL SUPERSTRUCTURE (GRADE 50W)	LS	1	
	NEOPRENE STRIP SEAL DAM	LF	63	
	TEMPORARY SHORING	LS	1	
207032-000	FABRIC FOR SUBSURFACE DRAINAGE	SY	170	
212001-000	STRUCTURE EXCAVATION	CY	825	
212005-000	SELECT MATERIAL FOR BACKFILLING	CY	59	
218006-000	FOUNDATION PROTECTION	CY	200	
601002-001	CLASS B CONCRETE	CY	225	
601003-001	CLASS K CONCRETE	CY	22	
601009-001	CLASS H CONCRETE	CY	130	
602001-001	REINFORCING STEEL BAR	LB	17500	
602002-001	EPOXY COATED REINFORCING STEEL BAR	LB	24000	
615029-001	NON-GUIDED BEARING, ELASTOMERIC	EA	13	
615030-001	FIXED BEARING, ELASTOMERIC	EA	13	
614001-006	HP12X53 STEEL PILE, DRILLED	LF	500	
639001-001	CONSTRUCTION LAYOUT STAKE	LS	1	
	·			

ITEM 615001-001 STEEL		
SUPERSTRUCTURE		
(FOR INFORMATION ONLY)		
DESCRIPTION	UNIT	QUANTITY
STRUCTURAL STEEL, AASHTO M270, GRADE 50W	-	-
GIRDERS	LB	74532
DIAPHRAGMS	LB	12348
STIFFENERS AND CONNECTION PLATES	LB	3244
STUD SHEAR CONNECTORS (# INCH DIA X 41/2 INCH)	LB	1094
HIGH STRENGTH BOLTS (# INCH DIA), AASHTO M164	LB L	605



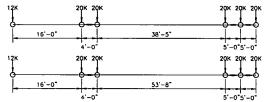
DOMINION TRANSMISSION, INC. LL TONKIN COMPRESSOR STATION BRIDGE DODDRIDGE COUNTY, WEST VIRGINIA

Civil & Environmental Consultants, Inc. 600 Marketplace Ave · Suite 200 · Bridgeport, WV 26330 Ph.: 304.933.3119 · 855.488.9539 · Fax: 304.933.3327 www.cecinc.com

SUMMARY OF QUANTITIES AND SHEET INDEX

4 4 OF 30

DESIGN LOAD ALL MEMBERS ARE DESIGNED FOR AN HL-93 LIVE LOAD. THE DESIGN PROVIDES FOR AN ADDITIONAL OF 25 P.S.F. FUTURE WEARING SURFACE (NOT IN THIS CONTRACT) AND FOR PERMANENT METAL DECK FORMS (15 P.S.F. INCLUDING CONCRÉTE IN FLUTES.



THE ABOVE LIVE LOADS ARE ANALYZED FOR OPERATING RATING ONLY

DESIGN METHOD
THE BRIDGE DESIGN IS IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE
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MATERIAL DESIGN STRENGTHS	
CLASS K REINFORCED CONCRETE	F'c = 4,000  p.s.i
CLASS B REINFORCED CONCRETE	F'c = 3,000  p.s.i
CLASS H REINFORCED CONCRETE	F'c = 4,000  p.s.i.
REINFORCING STEEL	Fy = 60,000  p.s.i.
STRUCTURAL STEEL	Fy = 50,000  p.s.i.
	Fv = 70.000  p.s.i.

MATERIAL SPECIFICATIONS
ASSHTO LRFD BRIDGE DESIGN SPECIFICATIONS, SEVENTH EDITION 2014, AS DESIGNATED BELOW SHALL GOVERN THE MATERIALS FURNISHED, UNLESS OTHERWISE NOTED

AASHTO M31
AASHTO M284
AASHTO M270 - GRADE 50
AASHTO M314
AASHTO M164, TYPE 3
GRADE 3, 60-DUROMETER

CONCRETE
CLASS H CONCRETE IS TO BE USED IN THE SUPERSTRUCTURE SLAB. CLASS CONCRETE IS TO BE USED IN THE PARAPETS. CLASS B CONCRETE IS TO BE USED IN THE ABUTMENTS, WINGWALLS, AND NEWLY CONSTRUCTED PORTIONS OF THE PIERS. CONTRACTOR'S ATTENTION IS CALLED TO THE TEST REQUIREMENTS FOR THE ADMIXTURES. STAY—IN—PLACE METAL FORMS SHALL BE USED ON THIS PROJECT. REMOVABLE FORMS SHALL BE USED ON THE OVERHANGS. PAYMENT FOR FORMWORK SHALL BE INCLUDED IN THE CLASS H CONCRETE BID ITEM.

# STRUCTURAL STEEL

ALL STRUCTURAL STEEL USED IN THE GIRDER FLANGES, WEBS, DIAPHRAGMS, STIFFENERS AND CONNECTION PLATES SHALL MEET THE CHARPY V-NOTCH REQUIREMENTS OF AASHTO ZONE 2.

ANCHOR BOLTS, NUTS AND WASHERS MAY BE MANUFACTURED FROM ORDINARY MILD STEEL AND SHALL BE HOT-DIP GALVANIZED ACCORDING TO AASHTO M111 AFTER FABRICATION. THE FABRICATOR'S SHOP DRAWINGS SHALL IDENTIFY THE MATERIAL SPECIFICATION AND GRADE FOR EACH ITEM AND ARE SUBJECT TO APPROVAL OF THE ENGINEER

HIGH STRENGTH FASTENERS SHALL MEET SECTION 709.24 AND SHALL BE BLACK (UNCOATED) TYPE 3 (WEATHERING STEEL). THE HIGH STRENGTH FASTENERS USED IN REGIONS OF THE STRUCTURE THAT REQUIRE PAINTING SHALL BE MECHANICALLY GALVANIZED.

BEFORE ASSEMBLING THE HIGH STRENGTH BOLTED CONNECTIONS, REMOVE ALL LOOSE AND NON-ADHERENT RUST THAT MAY HAVE FORMED ON THE CONNECTION AREAS BY HAND OR POWER WIRE BRUSHING

ALL STRUCTURAL CONCRETE SHALL BE PLACED, CURED AND FINISHED IN ACCORDANCE WITH SECTION 601 OF THE SPECIFICATIONS.

BLAST CLEANING AND PAINTING UPON COMPLETION OF ALL FABRICATION OPERATIONS IN THE SHOP, AND DPON COMPLETION OF ALL FABRICATION OPERATIONS IN THE SHOP, AND BEFORE SHIPMENT TO THE PROJECT SITE, ALL WEATHERING STEEL BRIDGE COMPONENTS SHALL BE BLAST CLEANED TO A NEAR WHITE SURFACE CONDITIONS ACCORDING TO SSPC—SP 10. PRIOR TO THE START OF ANY BLAST CLEANING, ALL OIL, GREASE, CUTTING FLUIDS, OR OTHER FOREIGN MATTER SHALL BE REMOVED FROM THE SURFACES OF THE STEEL BY SOLVENT CLEANING ACCORDING TO SSPC-SP 1

THE MEMBERS OR PORTIONS OF MEMBERS LISTED BELOW SHALL BE BLAST CLEANED AND SHOP PAINTED ACCORDING TO SECTION 688 OF THE STANDARD SPECIFICATIONS, PAINTING STEEL STRUCTURE. USING THE ZINC RICH. LOW VOC SYSTEM, SECTION 711.20. APPLY THE FULL PAINT SYSTEM IN THE FABRICATION SHOP EXCEPT PAVING SURFACE OF HIGH STRENGTH BOLTED CONNECTIONS, WHICH SHALL BE SHOP PAINTED WITH PRIMER ONLY. THE COLOR OF THE FINAL TOP COAT SHALL BE 30045 ACCORDING TO FEDERAL STANDARD 595 AND THE GLOSS AT AN ANGLE OF 60 DEGREES SHALL NOT EXCEED 25.

PAINT THE ENDS OF THE GIRDERS AND ALL OTHER STRUCTURAL COMPONENTS WITHIN 5 FEET OF BEARINGS, EXCEPT THE TOP FACE OF THE TOP FLANGE WHICH SHALL BE PRIMED AT 2 MIL. DRY

AREAS OF THE SHOP APPLIED PAINT SYSTEM WHICH ARE DAMAGED DURING ERECTION AND HIGH STRENGTH BOLTED CONNECTION AREAS THAT WERE ONLY PRIME PAINTED SHALL BE PROPERLY CLEANED AND PAINTED ACCORDING TO OF THE STANDARD SPECIFICATIONS, AND TO THE SATISFACTION OF THE ENGINEER

BLAST CLEANING AND PAINTING (CONT.)
AFTER COMPLETION OF ALL TIGHTENING, GALVANIZED MECHANICAL FASTENERS
SHALL BE SOLVENT CLEANED AND FIELD PAINTED AS SPECIFIED FOR THE

INCLUDE CLEANING AND PAINTING COSTS IN THE STEEL SUPERSTRUCTURE

IDENTIFICATION MARKING STEEL MEMBERS
ALL STEEL MILL AND FABRICATOR IDENTIFICATION MARKINGS FOR STEEL PLATES, SHAPED, OR FABRICATED MEMBERS SHALL BE BY METAL TAGS, SOAPSTONE, OR SOME OTHER READILY REMOVABLE MATERIAL; OR, SHALL MARKED IN AN AREA OF COMPLETED MEMBER WHICH WILL BE ENCASED OR COVERED WITH CONCRETE. MARKING METHODS AND LOCATIONS ARE SUBJECT TO APPROVAL OF THE ENGINEER.

DO NOT USE PAINT OR WAX-BASED CRAYONS FOR MARKING.

HANDLING AND STORING STEEL MEMBERS
EXTREME CARE SHALL BE EXERCISED IN HANDLING THE STEEL IN THE SHOP, DURING SHIPPING, ERECTION, AND SUBSEQUENT CONSTRUCTION OF THE BRIDGE. PAINTED STEEL SHALL NOT BE MOVED OR HANDLED UNTIL SUFFICIENT CURE TIME HAS ELAPSED TO ENSURE NO DAMAGE IS DONE TO THE FRESH COATING. THE STEEL SHALL BE INSULATED FROM THE BINDING CHAINS BY SOFTENERS. HOOKS AND SLINGS USED TO HOIST STEEL SHALL BE PADDED. TO PREVENT DAMAGE, STEEL MEMBERS AND COMPONENTS SHALL BE SPACED IN SUCH A WAY THAT NO RUBBING WILL OCCUR DURING

STEEL MEMBERS MUST NOT BE GOUGED, SCRATCHED, DENTED, OR ALLOWED TO RUB AGAINST OTHER MEMBERS THAT WOULD RESULT IN DAMAGE TO THE BLAST CLEANED PROFILE OF THE STEEL. MEMBERS SHALL BE HANDLED USING SOFTENERS AND SLINGS INSTEAD OF CHOKERS AND CHAINS.

STORE MEMBERS IN THE FABRICATION SHOP AND ON THE PROJECT SITE IN SUCH A MANNER AS TO BE KEPT FREE AND CLEAN OF ALL FOREIGN SUBSTANCES SUCH AS GREASE, OIL, MORTAR, CONCRETE SPLATTER, CHALK AND CRAYON MARKS, PAINT AND DIRT. ALL STORAGE MUST BE ABOVE GROUND AND SLOPED TO ALLOW FREE DRAINAGE OF MELTED SNOW, RAINWATER, AND DEW. IF STORED FOR PERIODS LONGER THAN 3 MONTHS, THE MEMBERS MUST BE PLACED ON METAL SUPPORTS. FOR PERIODS OF STORAGE UP TO 3 MONTHS, MEMBERS MAY BE PLACED ON CLEAN UNTREATED, WOOD TIMBERS.

STORE GIRDERS WITH THE WEB IN THE UPRIGHT POSITION. THE MEMBERS MAY BE STACKED PROVIDED METAL OR WOOD SUPPORTS, AS NOTED ABOVE, SEPARATE INDIVIDUAL MEMBERS. UNDER NO CIRCUMSTANCES SHALL MEMBERS BE NESTED TOGETHER OR BUNDLED

DO NOT ALLOW TREATED LUMBER OR TREATED TIMBER TO CONTACT STEEL CONTACT WITH CLEAN, UNTREATED LUMBER OR TIMBER WILL NOT DAMAGE THE STEEL MEMBERS.

FINAL CLEANUP OF STRUCTURAL STEEL SURFACES
UPON COMPLETION OF ALL CONCRETE CURING OPERATIONS, THE
CONTRACTOR SHALL CLEAN ALL STEEL SURFACES TO REMOVE ALL GREASE,
OIL, CONCRETE RESIDUE, DIRT, AND OTHER FOREIGN SUBSTANCES TO THE SATISFACTION OF THE ENGINEER.

CLEANING MAY BE BY HIGH PRESSURE WATER POWER OR HAND WIRE BRUSHING, OR BY BRUSH-OFF BLAST CLEANING ACCORDING TO SSPC-SP 7.
CLEANING SHALL BE FOLLOWED BY A CLEAN WATER RINSE TO REMOVE ALL RESIDUES OF DETERGENTS AND CLEANERS. ALL GREASE AND OIL SHALL BE REMOVED PRIOR TO THE CLEAN WATER RINSE BY SOLVENT CLEANING. DO NOT USE ACIDS TO REMOVE STAINS.

INCLUDE COSTS FOR FINAL CLEANUP OF STEEL SURFACES IN THE STEEL SUPERSTRUCTURE BID ITEM.

PROTECTION OF CONCRETE SUPERSTRUCTURE
AFTER CONCRETE HAS ACHIEVED A MINIMUM OF 80% OF THE DESIGN
STRENGTH, A SILANE TREATMENT SHALL BE APPLIED TO THE ENTIRE TOP SURFACE OF THE BRIDGE DECK AND AROUND THE PARAPETS TO THE DRIP NOTCH ON THE UNDERSIDE OF THE DECK. THE SILANE PROTECTIVE TREATMENT SHALL BE INCLUDED IN THE CLASS H CONCRETE BID ITEM

PROTECTION OF CONCRETE SUBSTRUCTURE BEFORE PLACING ANY STEEL SUPERSTRUCTURE MEMBERS ON THE CONCRETE SUBSTRUCTURE UNITS, THE CONTRACTOR SHALL COAT ALL EXPOSED AREAS OF THE ABUTMENTS AND PIERS, TOPS AND SIDES, TO THE GROUND OR WATER LINE ELEVATIONS WITH AN APPROVED SILANE BASED CONCRETE SEALER. PREPARATION OF SURFACES, APPLICATION RATES, AND METHODS SHALL BE AS RECOMMENDED BY THE SILANE MANUFACTURER

THE CONTRACTOR SHALL TAKE APPROPRIATE MEASURES TO PROTECT THE CONCRETE SUBSTRUCTURE FROM RUST STAINING DURING CONSTRUCTION AND CURING OF SUPERSTRUCTURE CONCRETE. WATER RUNOFF FROM CONCRETE CURING OPERATIONS SHALL BE DEFLECTED AWAY FROM STEEL GIRDERS AND SHALL NOT DRAIN ONTO THE SUBSTRUCTURE CONCRETE AFTER CONTACTING THE WEATHERING STEEL.

UPON COMPLETION OF ALL SUPERSTRUCTURE CONCRETE CURING OPERATIONS, THE CONTRACTOR SHALL REMOVE ALL RUST STAINS FROM SUBSTRUCTURE UNITS USING PROPRIETARY CHEMICAL STAIN REMOVERS OR MILD ACID ETCHING. ABRASIVE BLAST CLEANING MAY BE USED TO SUPPLEMENT THE OTHER CLEANING METHODS IF THE STAINED AREAS ARE SEVERE OR EXTENSIVE. ALL CLEANING METHODS SHALL BE SUBJECT TO APPROVAL OF THE ENGINEER.

RE-COAT SUBSTRUCTURE CONCRETE AT ALL AREAS WHERE RUST STAINS WERE REMOVED, REGARDLESS OF THE CLEANING METHOD USED, WITH AN APPROVED SILANE BASED CONCRETE SEALER AS SPECIFIED ABOVE.

INCLUDE THE COST OF SILANE COATING, PROTECTING, CLEANING, AND RE-COATING SUBSTRUCTURE UNITS IN THE CLASS B CONCRETE BID ITEM

MEMBRANE WATERPROOFING SHALL BE APPLIED TO THE SURFACES OF THE ABUTMENTS AND WINGWALLS IN CONTACT WITH SOIL. FOOTERS/PILE CAPS DO NOT REQUIRE MEMBRANE WATERPROOFING. THE COST OF THE MEMBRANE WATERPROOFING SHALL BE INCLUDED IN THE CLASS B CONCRETE BID ITEM.

STEEL STUD SHEAR CONNECTORS
NO SHOP INSTALLATION OF WELDED STUD SHEAR CONNECTORS SHALL BE

THE CONTRACTOR MAY SHOP INSTALL SHEAR STUDS TO THE BEAM OR GIRDER FOR THE PURPOSE OF ATTACHING A WORKER FALL PROTECTION SYSTEM. ONLY THE SHEAR STUDS REQUIRED TO PROPERLY INSTALL AND SUPPORT THE WORKER FALL PROTECTION SYSTEM MAY BE INSTALLED IN THE SHOP. THE REMAINING SHEAR STUDS FOR AN INDIVIDUAL ROW ACROSS THE BEAM OR GIRDER FLANGE MAY ALSO BE INSTALLED IN THE SHOP. ALL SHOP INSTALLED SHEAR STUDS SHALL BE SHOWN ON THE SHOP DRAWINGS.

THE FIELD INSTALLATION OF THE WELDED STUD SHEAR CONNECTORS SHALL NOT COMMENCE PRIOR TO THE INSTALLATION OF THE DECK FORMS IN THE AREA SURROUNDING THE WELDED STUD SHEAR CONNECTORS. OVERHANGING DECK FORMS MAY BE INSTALLED AFTER THE WELDED STUD SHEAR CONNECTORS ARE INSTALLED. THE WELDED STUD SHEAR CONNECTORS AND DECK FORMS SHALL BE INSTALLED IN A SEQUENCE THAT PERMITS WORKERS ACCESS THROUGH THE DECK AREA WITHOUT WALKING THROUGH INSTALLED WELDED STUD SHEAR CONNECTORS. THE COST OF INSTALLING SHEAR ENDS SHALL BE INCLUDED IN THE STEEL SUPERSTRUCTURE BID ITEM.

ALL WELDED STUD SHEAR CONNECTORS SHALL CONFIRM TO SECTION 615.3.3 OF THE STANDARD SPECIFICATIONS AND MEET THE REQUIREMENTS OF SECTION 7 OF THE AASHTO/AWS D1.5M/D1.5: 2015 BRIDGE WELDING CODE.

# ERECTION REQUIREMENTS

CONTRACTOR'S ATTENTION IS DIRECTED TO SECTIONS 615.6, ERECTION AND 615.2.2, ERECTION DRAWINGS, OF THE STANDARD SPECIFICATIONS.

## CHAMFERED EDGES

CHAMFER ALL EXPOSED EDGES \*\* X \*\* ON SUPERSTRUCTURE AND SUBSTRUCTURE COMPONENTS, UNLESS OTHERWISE NOTED.

ALL REINFORCING BARS SHALL BE LAPPED AS SHOWN ON THE PLANS. THE MINIMUM LAP LENGTH SHALL BE NO LESS THAN 30 BAR DIAMETERS. EMBEDMENT DIMENSIONS FOR REINFORCING BARS ARE CLEAR DIMENSIONS JNLESS OTHERWISE SHOWN OR NOTED. BAR SPACING IS GIVEN TO CENTERLINE OF THE BARS. UNLESS OTHERWISE SHOWN, THE CLEAR DISTANCE FROM FACE OF CONCRETE TO BARS SHALL BE AS FOLLOWS:

2 1/2" TOP SLAB

BOTTOM SLAB BOTTOM OF FOOTING

ELSEWHERE UNLESS NOTED OTHERWISE

THE REINFORCEMENT BAR SUFFIX "E" DESIGNATES EPOXY COATED BARS. ALL BARS IN SLAB, PARAPETS, AND ABUTMENTS ABOVE THE BEAM SEAT SHALL BE EPOXY COATED. PAYMENT WILL BE MADE UNDER THE BID ITEM FOR EPOXY COATED REINFORCING STEEL BAR. ALL OTHER REINFORCING STEEL BARS ARE UNCOATED AND ARE INCLUDED UNDER THE BID ITEM FOR REINFORCING

REINFORCING BARS UNDER BEARING LOCATIONS SHALL BE PLACED SO AS TO AVOID INTERFERENCE WITH ANCHOR BOLTS AND OTHER REINFORCING BARS.

IEMPORARY SUPPORTS
TEMPORARY SUPPORTS OR SHORING WILL BE REQUIRED UNDER THE BEAMS AT THE ABUTMENTS UNTIL CROSS FRAMES ARE INSTALLED. TEMPORARY SUPPORTS ARE TO BE REMOVED BEFORE ANY DECK CONCRETE IS PLACED. SCREED ELEVATIONS WILL BE TAKEN WITHOUT TEMPORARY SUPPORTS. THE TEMPORARY SUPPORT WILL BE PROVIDED AT NO ADDITIONAL COST.

FORM SUPPORTS SYSTEMS THAT WILL CAUSE OVER STRESS OR DEFORMATION TO PERMANENT BRIDGE MEMBERS WILL NOT BE PERMITTED. THE CONTRACTOR SHALL SUBMIT CALCULATIONS VERIFYING THAT THIS REQUIREMENT IS SATISFIED.

DECK SLAB OVERHANG FORMS SHALL BE SUPPORTED FROM THE BOTTOM FLANGE OF THE FASCIA GIRDERS. THE CONTRACTOR SHALL SUBMIT FORMING PLANS FOR THE OVERHANG TO THE ENGINEER PRIOR TO ERECTING THE FORMWORK. CONTRACTOR SHALL NOT COMMENCE WORK WITHOUT WRITTEN APPROVAL FROM ENGINEER.

THE AREA OF STAY-IN-PLACE FORMS IS 3,379 SF. THE CALCULATED VOLUME OF CLASS H CONCRETE INCLUDES 1 INCH OF CONCRETE OVER THE TOTAL AREA OF STAY-IN-PLACE FORMS FOR CONCRETE IN THE FORM FLUTES.

GEOTECHNICAL REPORT THE CONTRACTOR IS ADVISED TO REVIEW THE GEOTECHNICAL REPORT TITLED "DTI LL TONKIN COMPRESSOR STATION - WEST UNION, DODDRIDGE COUNTY, WEST VIRGINIA", AND DATED "JUNE 22ND, 2015".

ENVIRONMENTAL NO WORK SHALL OCCUR BELOW THE ORDINARY HIGH WATER ELEVATION OF MIDDLE ISLAND CREEK.

ELASTOMERIC BEARING WITH LOAD PLATE EXERCISE CAUTION WHILE MAKING WELDS WHILE AN ELASTOMERIC BEARING PAD IS IN CONTACT WITH THE STEEL. IN NO CASE SHALL THE ELASTOMER OR ELASTOMER BAND BE EXPOSED TO

INSTANTANEOUS TEMPERATURES GREATER THAN 400°F. ANY DAMAGE TO THE ELASTOMERIC BEARING DUE TO WELDING WILL BE CAUSE FOR REJECTION. THE TEMPERATURE SHALL BE MONITORED BY HEAT CRAYONS FURNISHED BY THE CONTRACTOR

<u>PILING</u> ALL PILES SHOWN IN THE PLANS SHALL CONFORM TO AASHTO M270 GRADE 50.

PILES ARE DESIGNED FOR THE FOLLOWING MAXIMUM AXIAL CAPACITIES WHICH ARE BASED ON USING A RESISTANCE FACTOR OF 0.50 AND GRADE 50 PILE

HP12x53 - 255 KIPS PER PILE

PILE DRIVING SHALL BE IN ACCORDANCE WITH SECTION 616 OF THE SPECIFICATIONS.

PILE SHALL BE DRIVEN WITH A VARIATION OF NOT MORE THAN 2% FROM THE VERTICAL OR FROM THE BATTER LINE INDICATED.

THE LOCATION IN PLAN FOR PILES IN FOUNDATION MAY HAVE A MAXIMUM DEVIATION OF 6 INCHES FROM THAT SHOWN IN THE CONTRACT DOCUMENTS.

ALL PILES SHALL BE DRIVEN TO REFUSAL. REFUSAL IS DEFINED IN BLOWS PER 1 INCH OF PENETRATION (BPI) TO THE TARGET BPI AS LISTED IN THE FOLLOWING TABLE, PROVIDED THAT DRIVING SHOULD BE TERMINATED IN ANY

EVENT AT 20 BLOWS PER 1 INCH TO AVOID DAMAGING THE PILES.

HAMMER TYPE	ENERGY RATING (FT-LBS)	FUEL SETTING	TARGET BPI	TARGET STROKE HEIGHT (FT.)
ICE 1-12	29625	MAX	11	9.6
ICE 42S	42004	MAX	10	10.4
PILECO D19-42	42506	MAX 2 (81%)	10	7.3
DELMAG D19-42	43240	MAX 2 (81%)	12	7.4
APE D19-32	42822	MAX 2 (81%)	7	7.6
APF D19-42	47126	MAX 2 (81%)	7	7.6

IF THE CONTRACTOR ELECTS TO USE A HAMMER THAT IS NOT INCLUDED IN THE ABOVE TABLE, OF IF A DIFFERENT FUEL SETTING THAN THOSE LISTED ABOVE IS USED, THEN THE CONTRACTOR SHALL SUBMIT DRIVING CRITERIA USING A WAVE EQUATION ANALYSIS, SUCH AS GRLWEAP. THE DRIVING CRITERIA SHALL BE SUBMITTED TO THE ENGINEER AT LEAST 14 DAYS PRIOR TO THE INSTALLATION OF THE FIRST PILE AND SHALL INCLUDE THE FUEL SETTING(S), STROKE HEIGHT, THE REQUIRED BLOWS PER 1 INCH OF PENETRATION TO ACHIEVE THE TARGET CAPACITY, AND THE MAXIMUM DRIVING STRESS REFUSAL. A MINIMUM OF 6 BPI MAY BE ALLOWED AS PRACTICAL REFUSAL BY THE ENGINEER BASED ON THE WAVE EQUATION ANALYSIS. THE ANALYSIS SHALL DEMONSTRATE THAT THE TARGET CAPACITY CAN BE ACHIEVED WITHOUT DAMAGING THE PILES OR EXCEEDING 45 KSI MAXIMUM COMPRESSIVE STRESS.

ANY WAVE EQUATION ANALYSIS PERFORMED BY THE CONTRACTOR AND ASSOCIATED COSTS DUE TO DELAYS, DRIVING EQUIPMENT OR MOBILIZATION SHALL BE AT THE CONTRACTORS EXPENSE.

BEFORE THE PILE DRIVING IS STARTED, THE CONTRACTOR SHALL PROVIDE WRITTEN CERTIFICATION TO THE ENGINEER THAT THE PILE HAMMERS. AIR COMPRESSORS AND AIR VALVES HAVE BEEN INSPECTED AND FOUND TO BE IN GOOD WORKING CONDITION. PILE POINTS ARE REQUIRED TO FACILITATE PILE DRIVING TO COMPETENT BEDROCK. COST OF TIPS TO BE INCLUDED IN PILING

SHOP DRAWINGS WILL BE SENT TO:

COPY TO:

ATTN: GREG LINDER, P.E.
CIVIL & ENVIRONMENTAL CONSULTANTS 600 MARKETPLACE AVE

ATTN: TIM GAWENUS DOMINION ENERGY 925 WHITE OAKS BLVD

# DEMOLITION PLAN

CONTRACTOR SHALL SUBMIT A PLAN OF REMOVAL/DEMOLITION OF THE EXISTING TRUSS BRIDGE AND THE EXISTING TEMPORARY BRIDGE. MATERIALS FROM THE EXISTING TRUSS BRIDGE SHALL BECOME THE PROPERTY OF THE CONTRACTOR

CONSTRUCTION OF WINGWALL B AND WINGWALL D MAY REQUIRE THE INSTALLATION OF TEMPORARY EXCAVATION SUPPORT AND PROTECTION SYSTEM DUE TO THE CLOSE PROXIMITY OF THE EXISTING TEMPORARY BRIDGE.

TEMPORARY EXCAVATION SUPPORT AND PROTECTION SYSTEM
A TEMPORARY EXCAVATION SUPPORT AND PROTECTION SYSTEM MAY BE
REQUIRED. THE CONTRACTOR IS RESPONSIBLE FOR SELECTING AND DESIGNING
THE TEMPORARY EXCAVATION SUPPORT AND PROTECTION SYSTEM AND MAY SELECT THE SUITABLE TYPE OF TEMPORARY SHORING SYSTEM THAT IS COMPATIBLE WITH THEIR CONSTRUCTION. THE TEMPORARY EXCAVATION SUPPORT AND PROTECTION SYSTEM DESIGN SHALL BE IN ACCORDANCE WITH THE CURRENT AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS AND CURRENT AASHTO GUIDE DESIGN SPECIFICATIONS FOR BRIDGE TEMPORARY WORKS. ALL WORK SHALL BE IN COMPLIANCE WITH OSHA PART 1926 SUBPART P. DESIGN THE TEMPORARY EXCAVATION SUPPORT AND PROTECTION SYSTEM FOR FINAL CONDITION AND ALL CONSTRUCTION CONDITIONS, INCLUDING SURCHARGE LOADS DUE TO VEHICLE TRAFFIC AND CONSTRUCTION EQUIPMENT. THE CONTRACTOR SHALL SUBMIT THE DESIGN CALCULATIONS AND COMPLETED DETAILED DRAWINGS, SIGNED AND SEALED BY A PROFESSIONAL ENGINEER, REGISTERED IN THE STATE OF WEST VIRGINIA. INCLUDE IN THE DESIGN CALCULATIONS ALL MATERIAL PROPERTIES, DESIGN LOADS, AND DESIGN ASSUMPTIONS. INCLUDE ON THE COMPLETED DETAILED DRAWINGS ALL DESIGN DIMENSIONS, LIMITS OF WORK, ELEVATIONS, MATERIAL, MEMBER SIZES AND CONSTRUCTION SEQUENCE. INCLUDE SPECINSTALLATION PROCEDURES AND TESTING REQUIREMENTS AS PART OF THE SUBMITTAL.

STANDARD DETAILS
THE WEST VIRGINIA DEPARTMENT OF TRANSPORTATION, DIVISION OF HIGHWAYS,
STANDARD DETAILS BOOK, VOLUME I, DATED MAY 1, 2016, VOLUME II, DATED
JANUARY 1, 1994, AND VOLUME III, DATED AUGUST 1, 1999 SHALL APPLY TO

THE WEST VIRGINIA DEPARTMENT OF TRANSPORTATION, DIVISION OF HIGHWAYS, TYPICAL SECTIONS AND RELATED DETAILS, DATED AUGUST 1, 2013 SHALL APPLY TO THIS PROJECT.

THE WEST VIRGINIA DEPARTMENT OF TRANSPORTATION, DIVISION OF HIGHWAYS EROSION AND SEDIMENT CONTROL MANUAL, DATED MARCH 1, 2003, AND AS AMENDED BY ADDENDUM 1 DATED OCTOBER 1, 2004 SHALL APPLY TO THIS

THE WEST VIRGINIA DEPARTMENT OF TRANSPORTATION, DIVISION OF HIGHWAYS, TRAFFIC ENGINEERING DIVISION, SIGN FABRICATION DETAILS, DATED SEPTEMBER 2005 SHALL APPLY TO THIS PROJECT.

ALL MANUALS LISTED ABOVE CAN BE FOUND AT:

http://www.transportation.wv.gov/highways/engineering/pages/publications.aspx

arketplace Ave 304.933.3119 · 8 Civil VIRGINIA ے∝

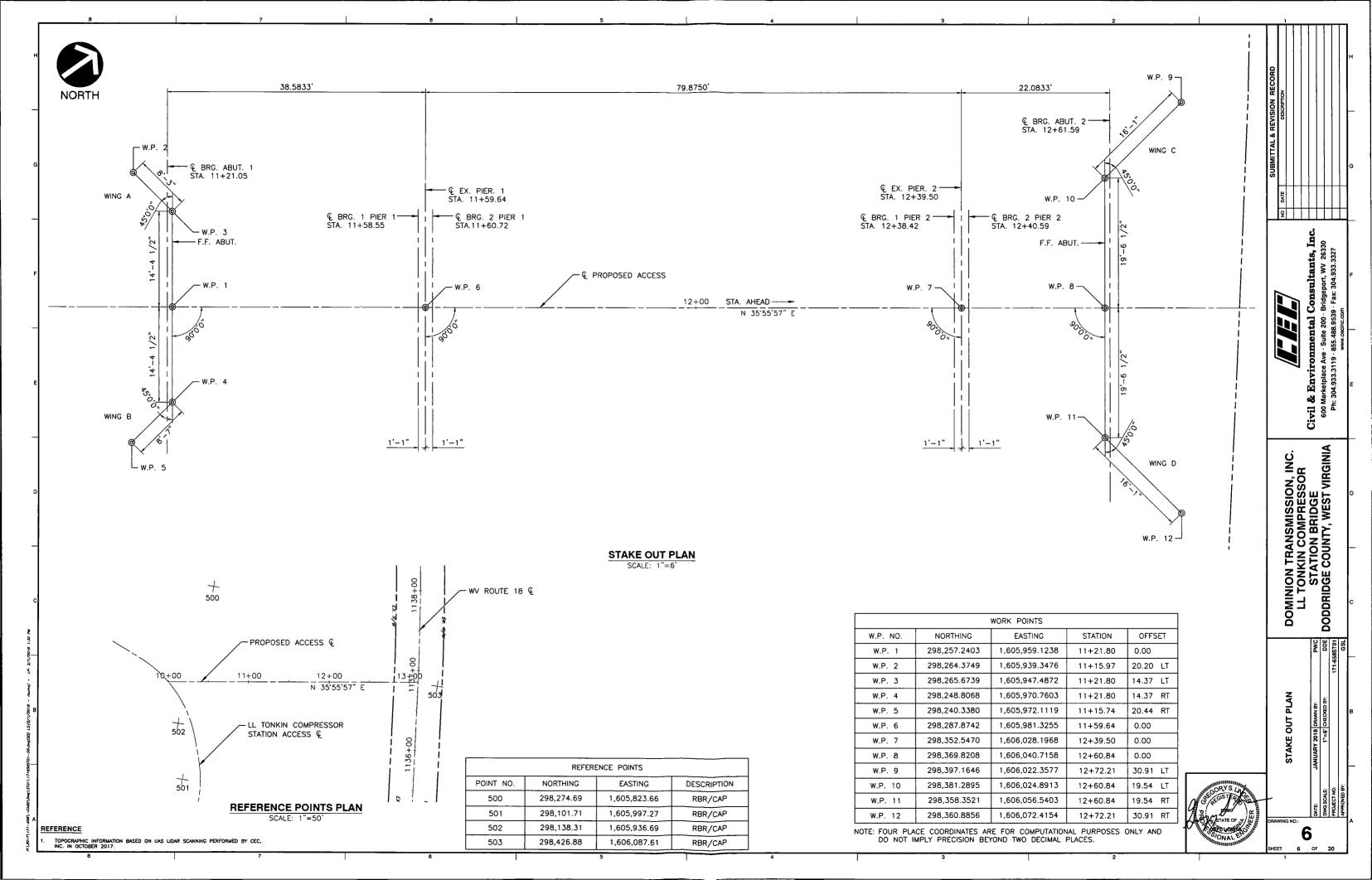
**sultan**1 port, WV 304.933.3

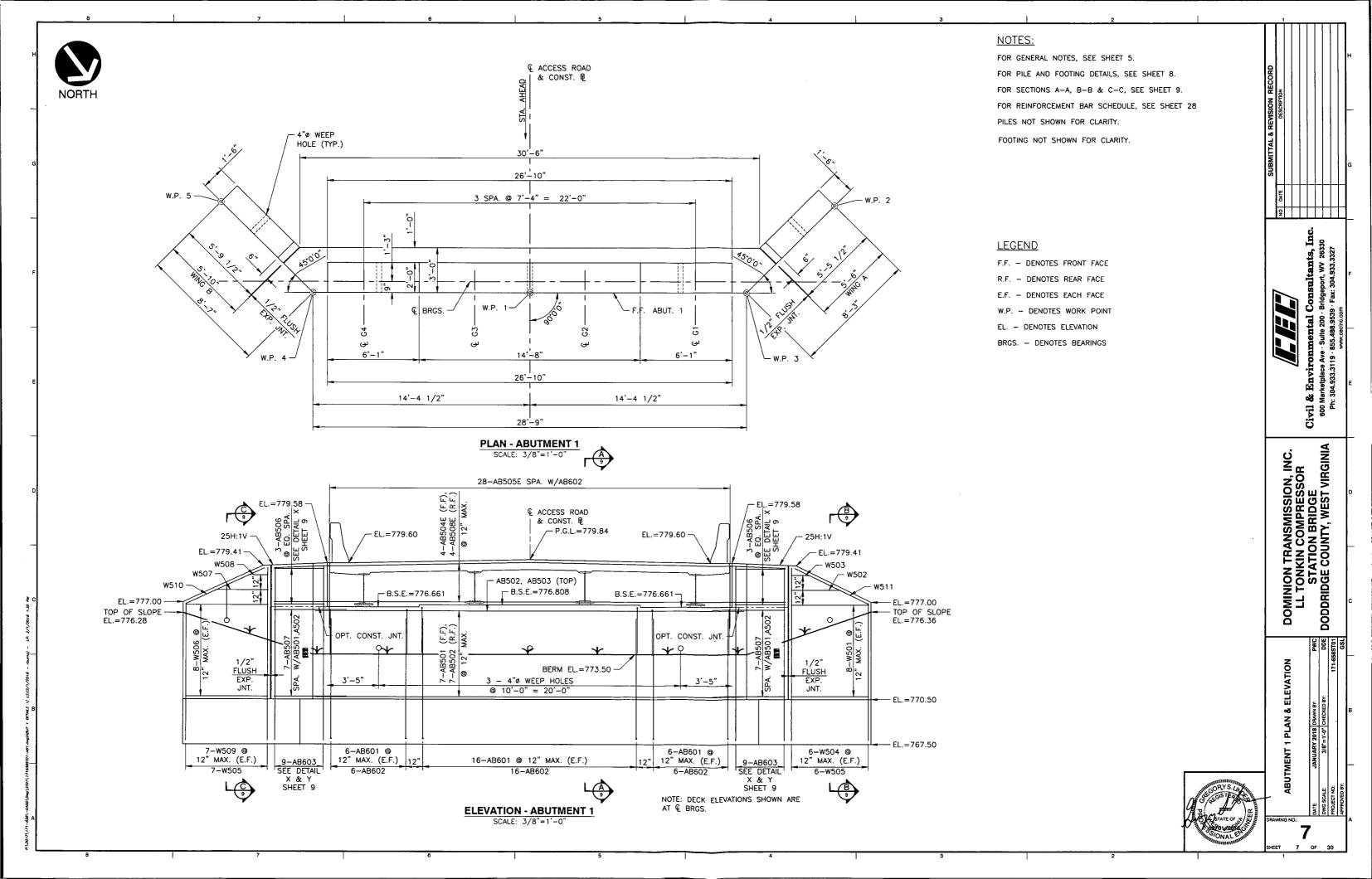
AISSION IPRESS RIDGE WEST V TATION BRII E COUNTY, W DOMINION LL TON ST, DODDRIDGE

PWC BST01

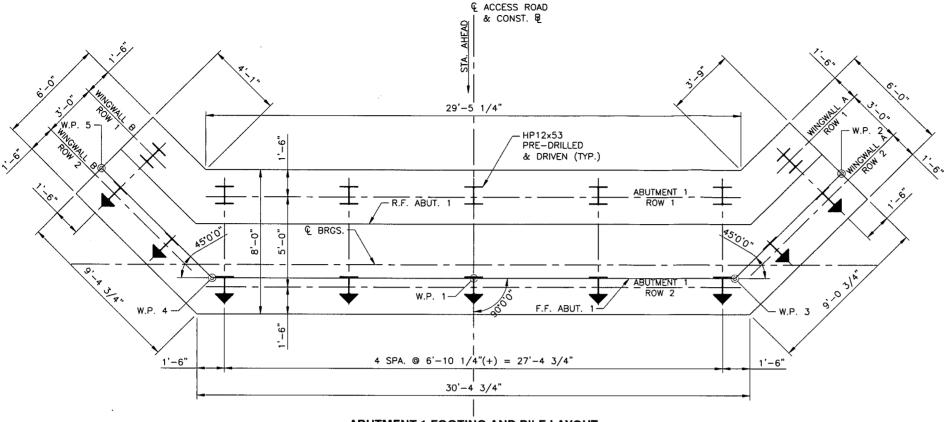
NOTES

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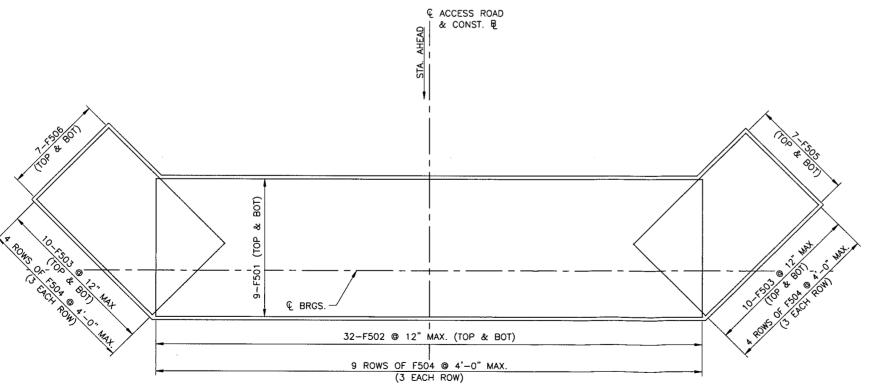








# **ABUTMENT 1 FOOTING AND PILE LAYOUT** SCALE: 3/8"=1'-0"



NOTE: SPACE FOOTING BARS TO CLEAR PILES.

# **ABUTMENT 1 FOOTING REINFORCEMENT PLAN**

SCALE: 3/8"=1'-0"

# NOTES:

FOR GENERAL NOTES, SEE SHEET 5.

FOR ABUTMENT PLAN & ELEVATION, SEE SHEET 7.

FOR REINFORCEMENT BAR SCHEDULE, SEE SHEET 28

ALL PILES AT ABUTMENT 1 SHALL BE PREDRILLED AND DRIVEN.

# **LEGEND**

F.F. - DENOTES FRONT FACE

R.F. - DENOTES REAR FACE

E.F. - DENOTES EACH FACE

W.P. - DENOTES WORK POINT

EL. - DENOTES ELEVATION

BRGS. - DENOTES BEARINGS

- HP12x53

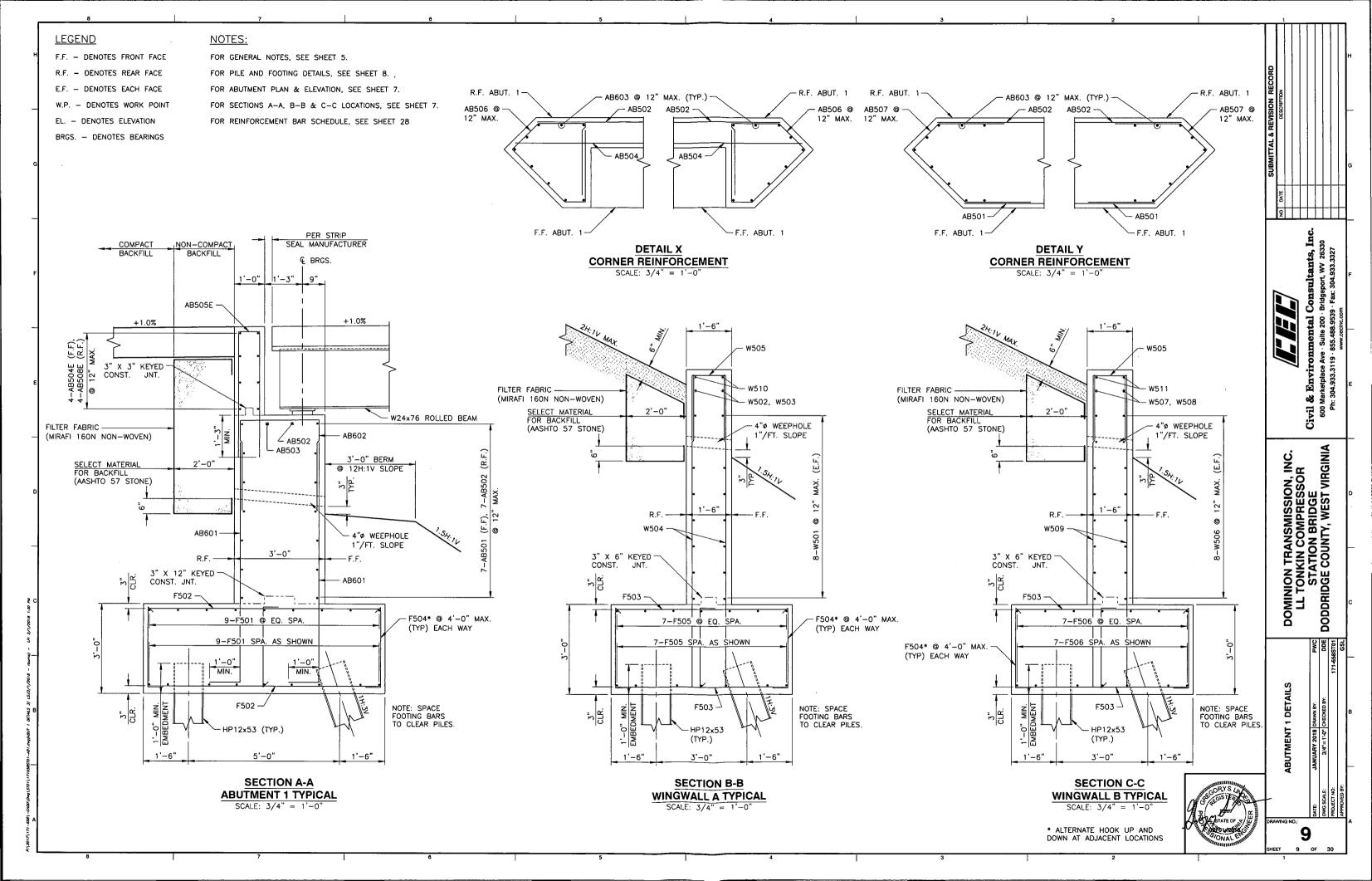
- HP12x53 BATTER 1H:3V IN DIRECTION OF ARROW

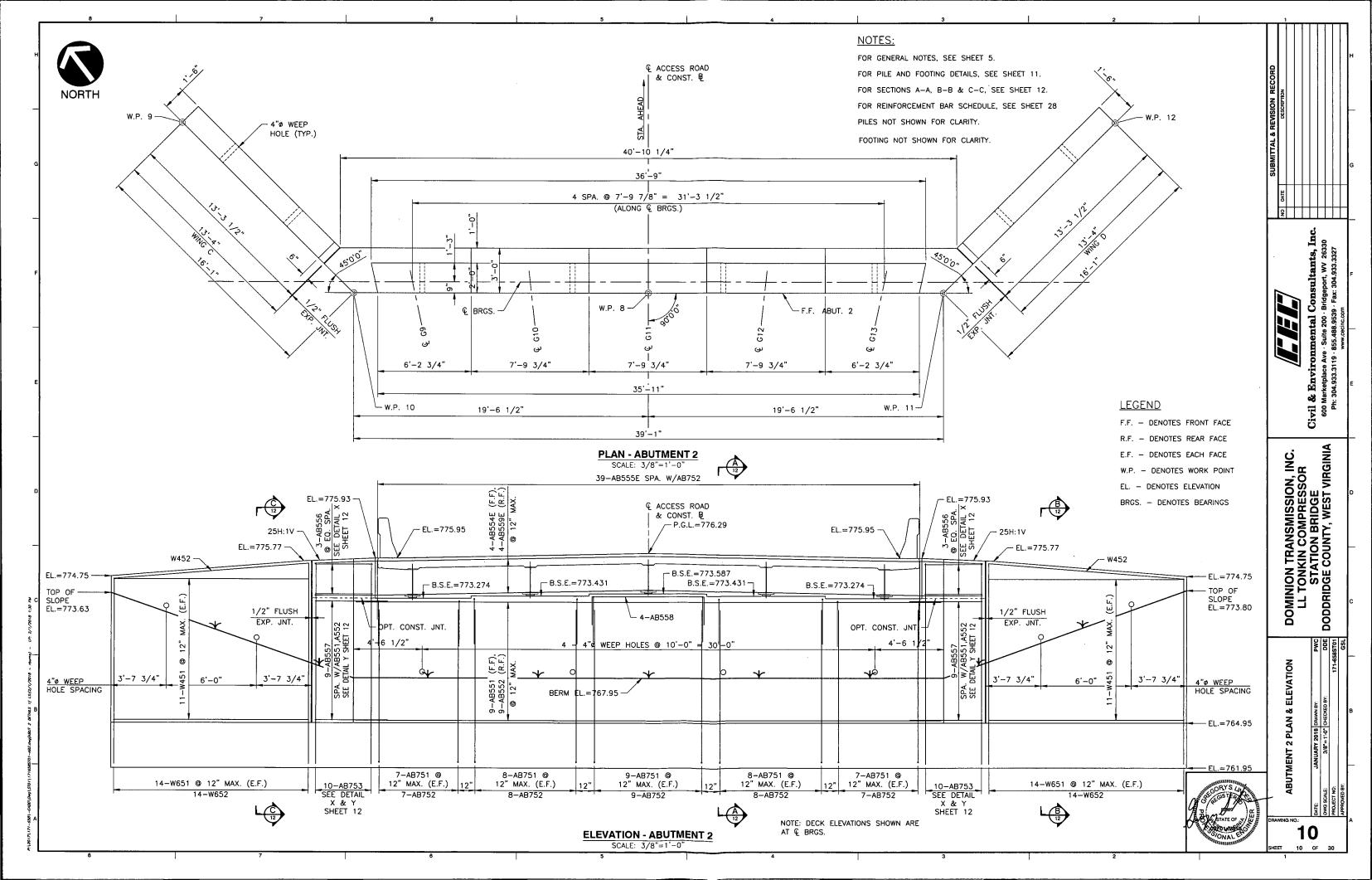
	PILE	SCHEDULE						
LOCATION	ROW	NO. PILES	SPACING					
ABUTMENT 1	1	5	6'-10 1/4" (+)					
ABUIMENT	2	5	6'-10 1/4" (+)					
WINGWALL A	1	1	N/A					
WINGWALL A	2	2	4'-0"					
WINGWALL B	1	1	N/A					
WINGWALL	2	2	4'-0"					

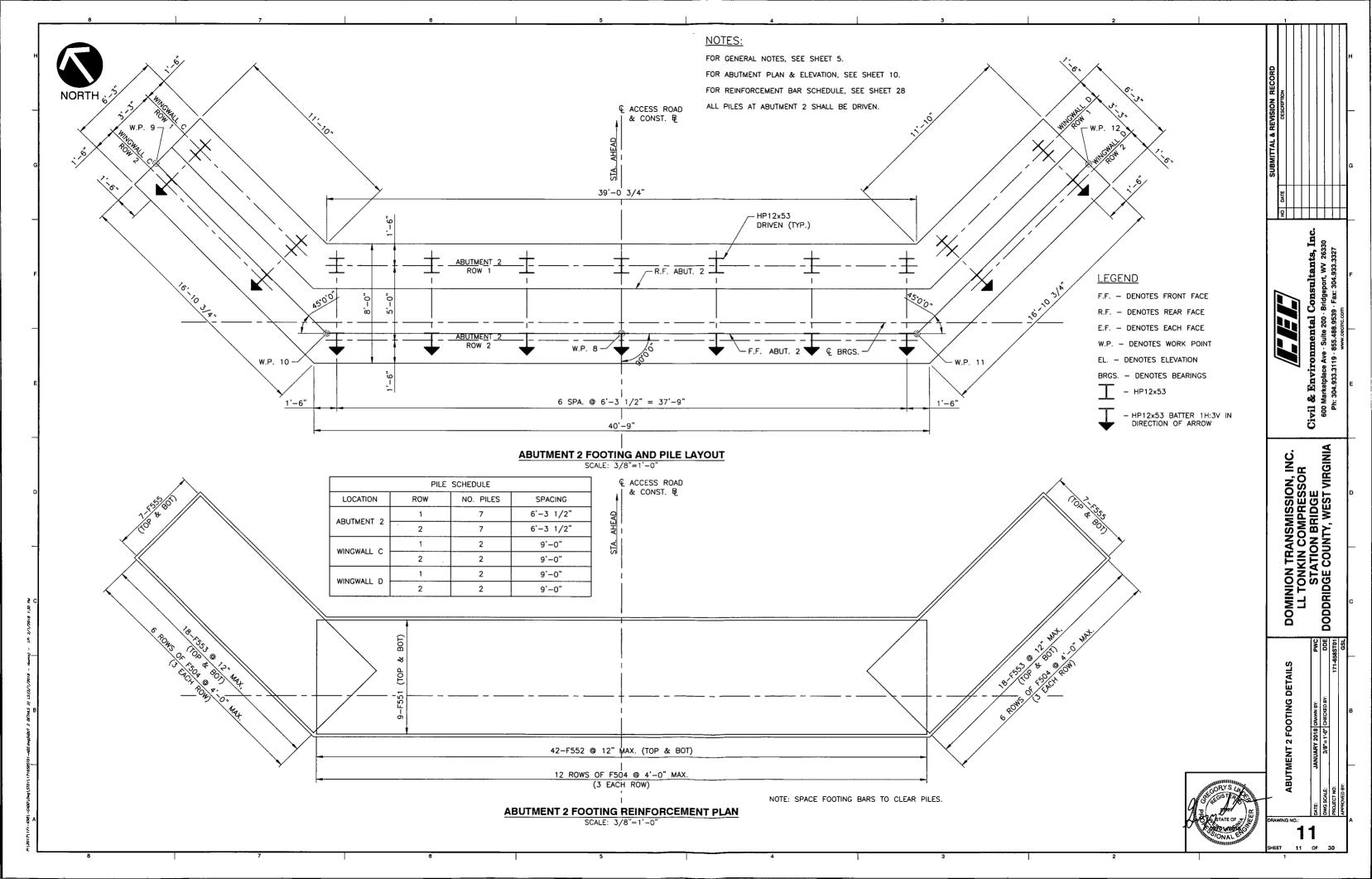


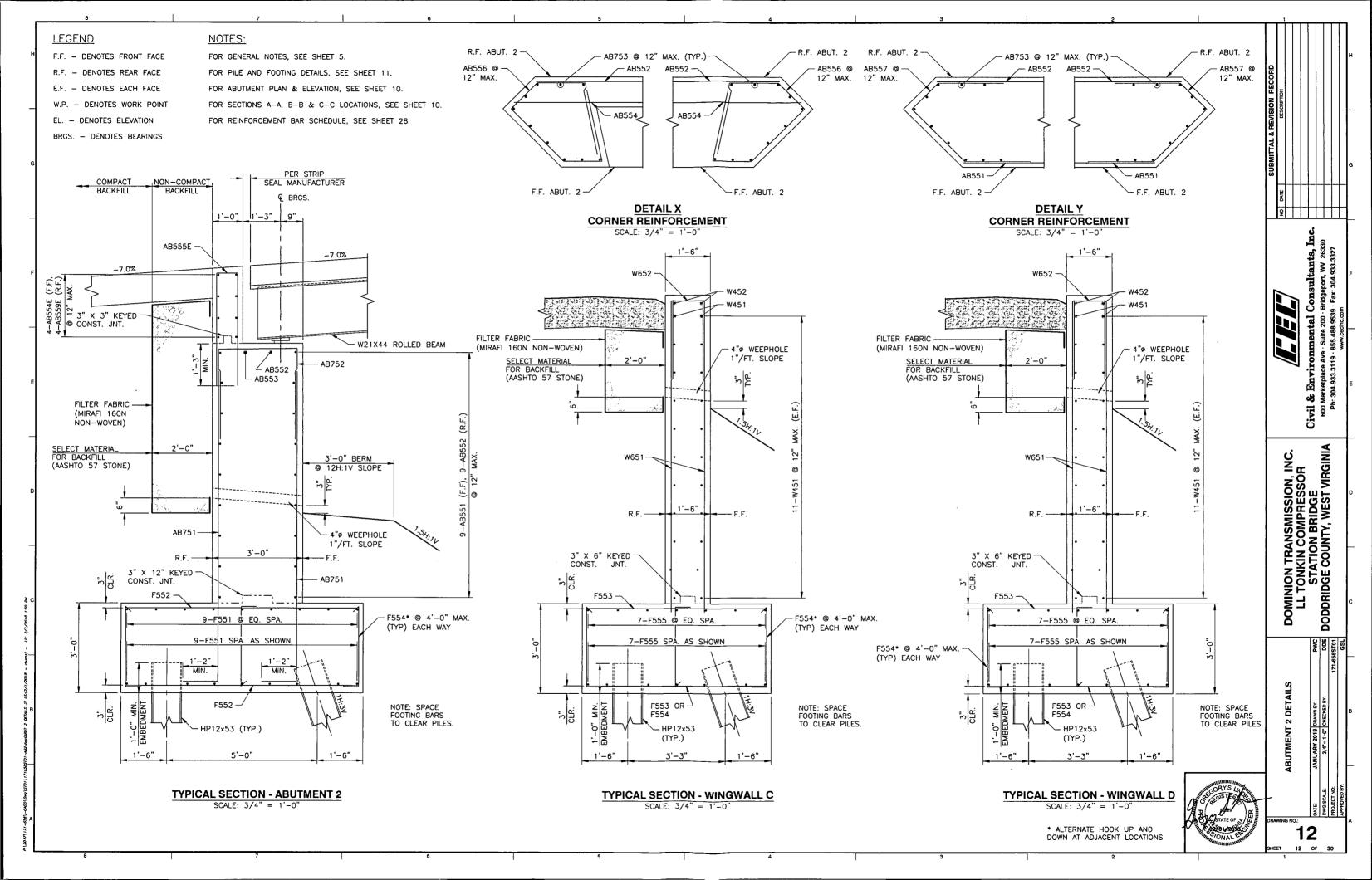
DOMINION TRANSMISSION, INC.
LL TONKIN COMPRESSOR
STATION BRIDGE
DODDRIDGE COUNTY, WEST VIRGINIA

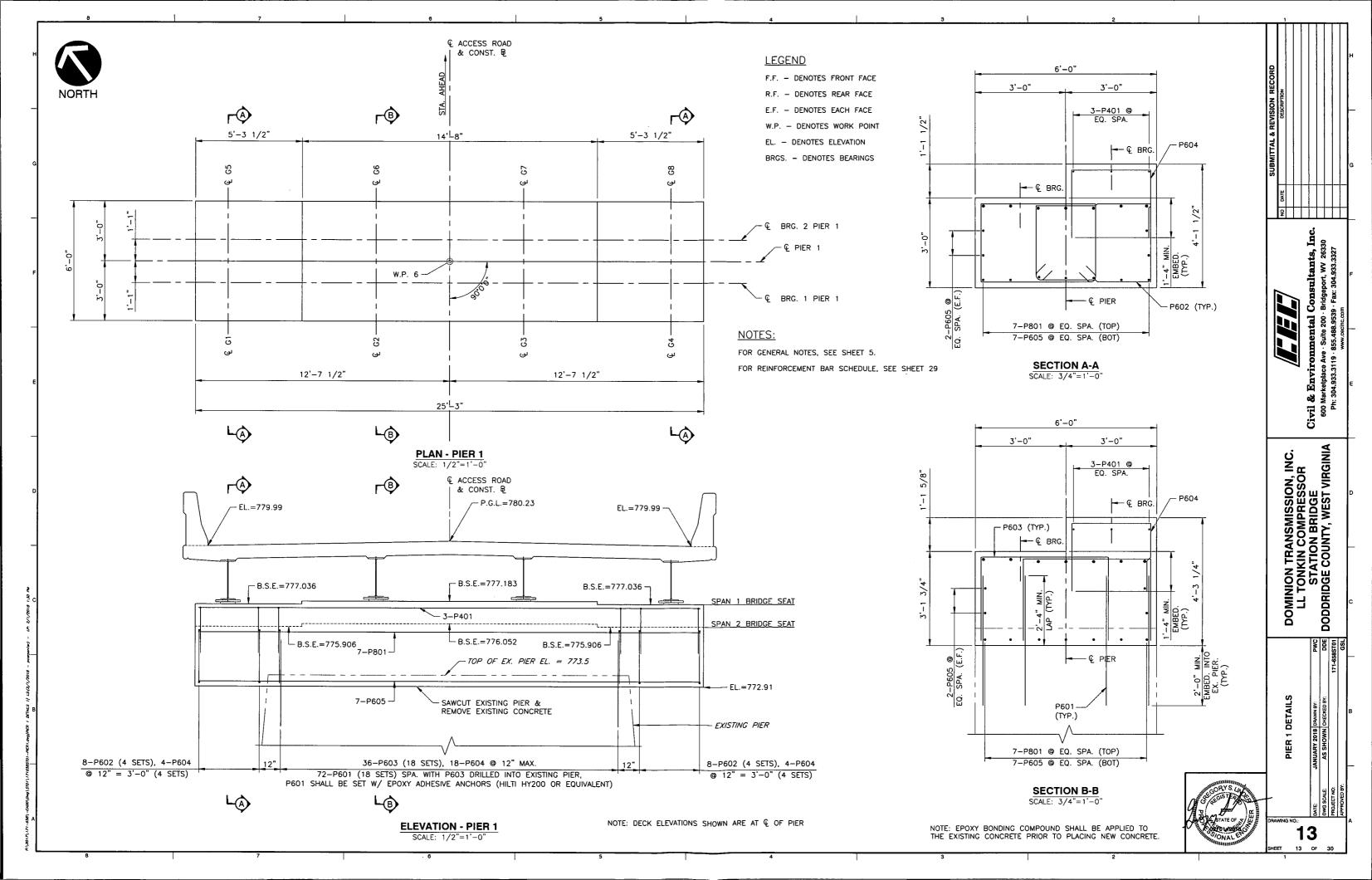
ABUTMENT 1 FOOTING DETAILS

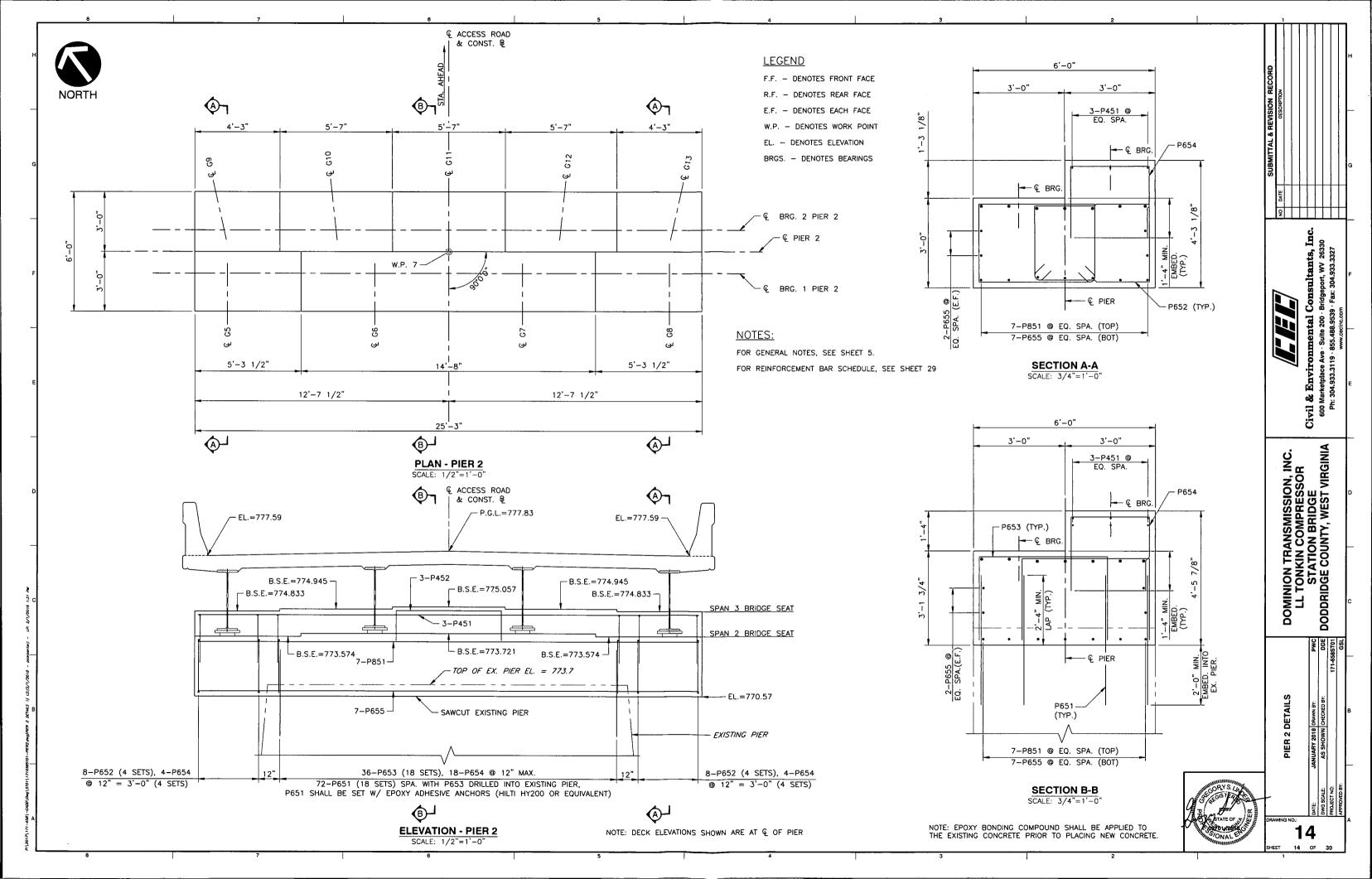


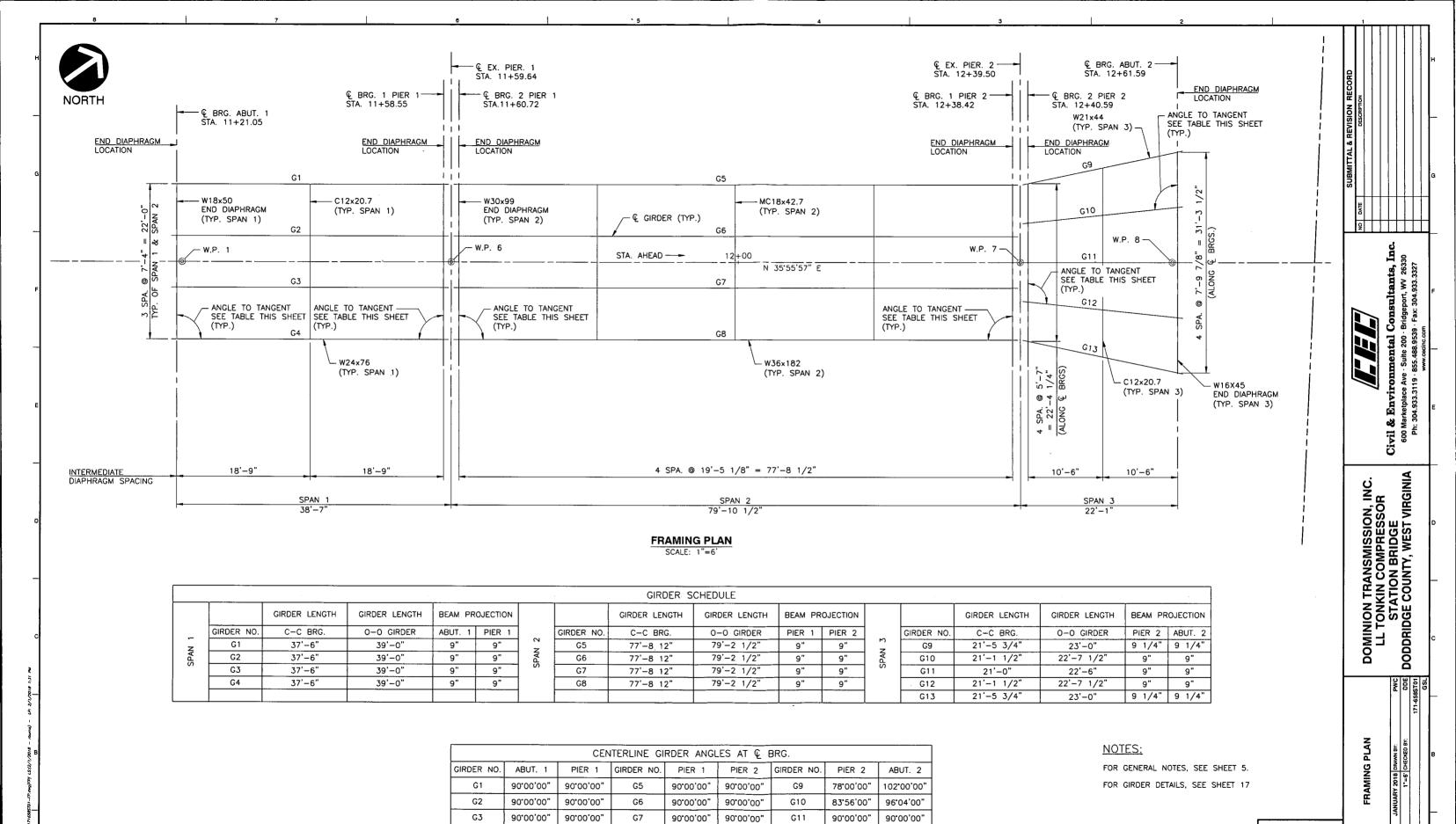












G12

G13

96'04'00"

102'00'00"

83.56,00,

78'00'00"

G4

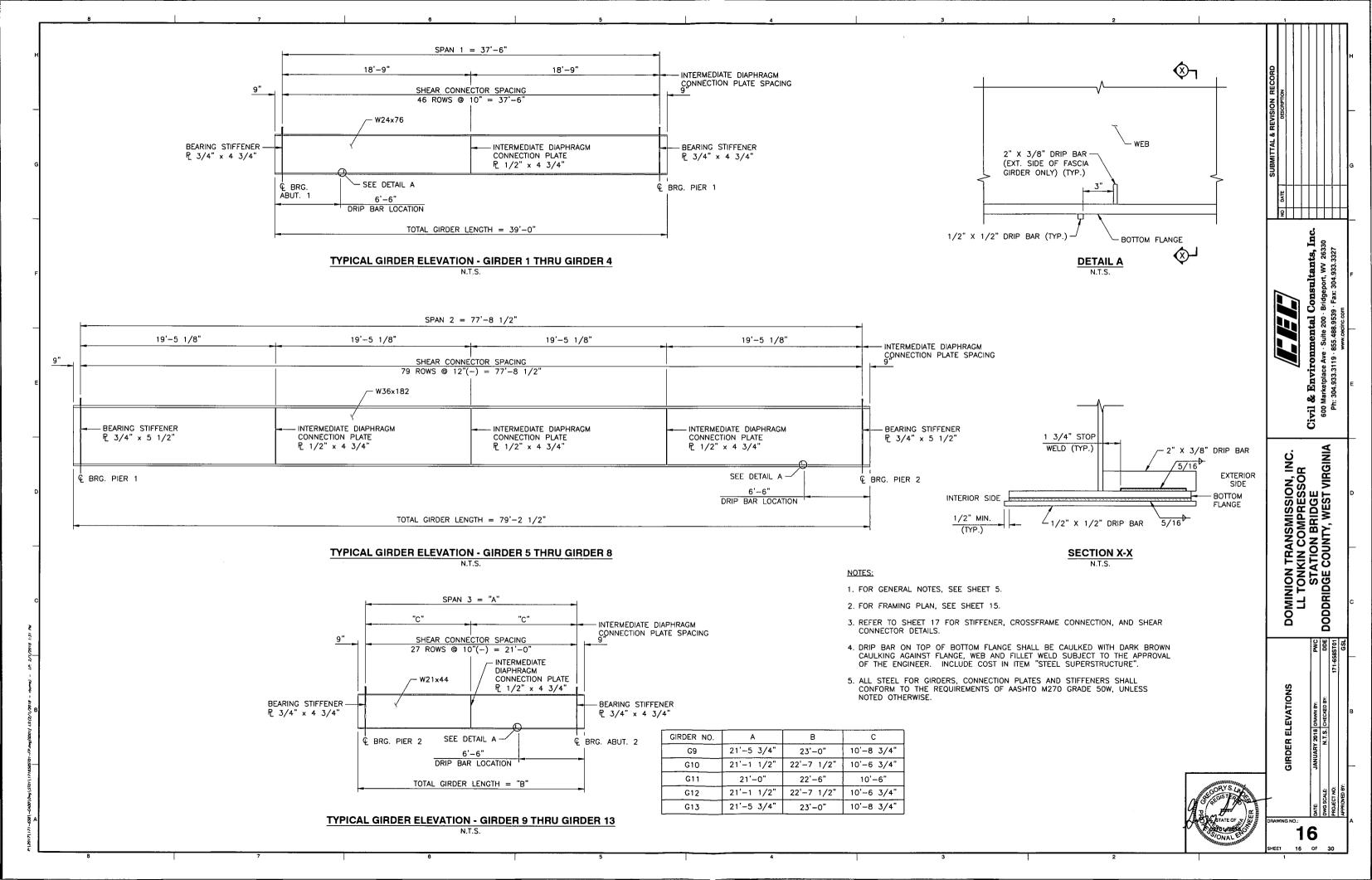
90,00,00,

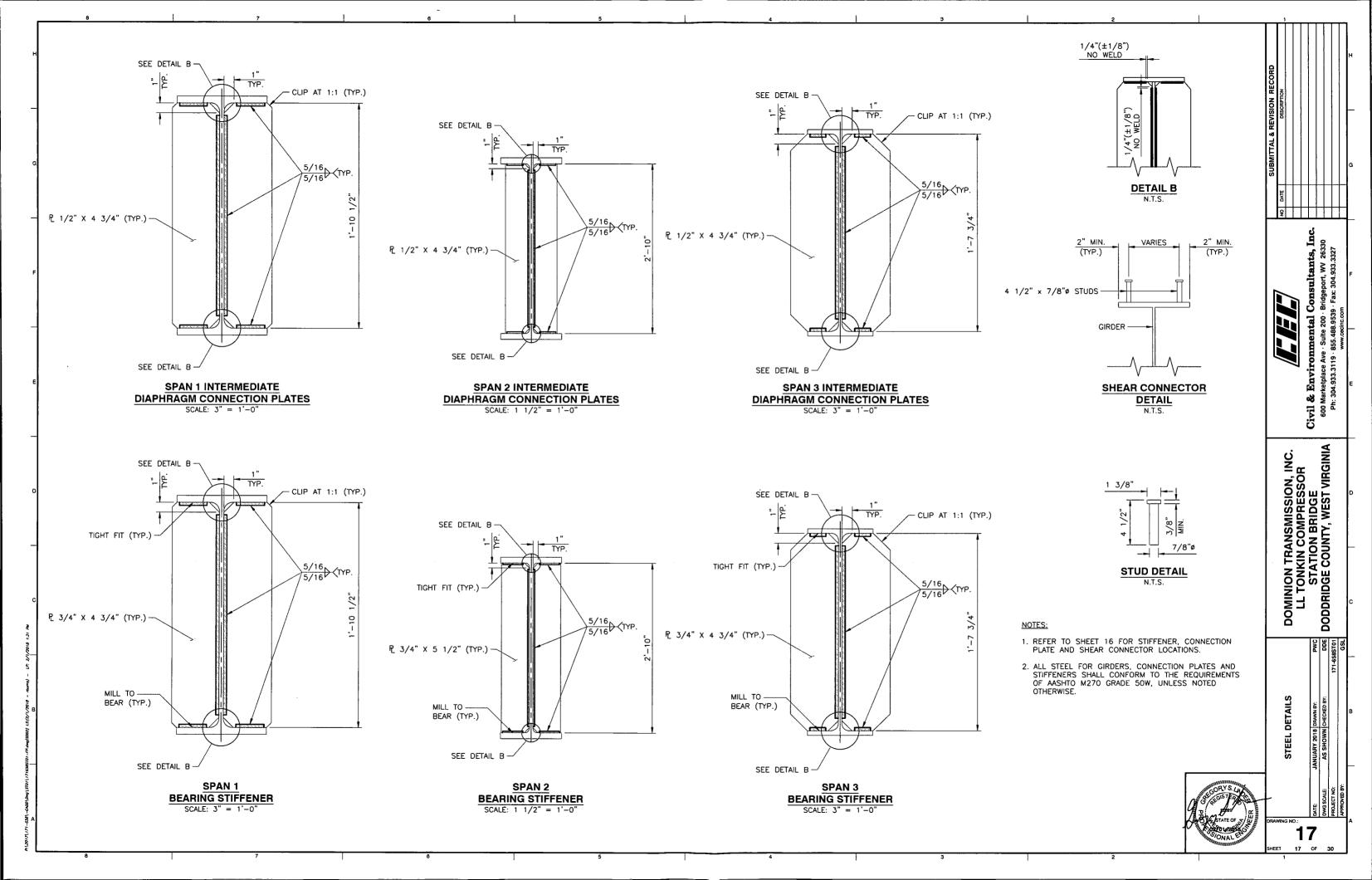
90'00'00"

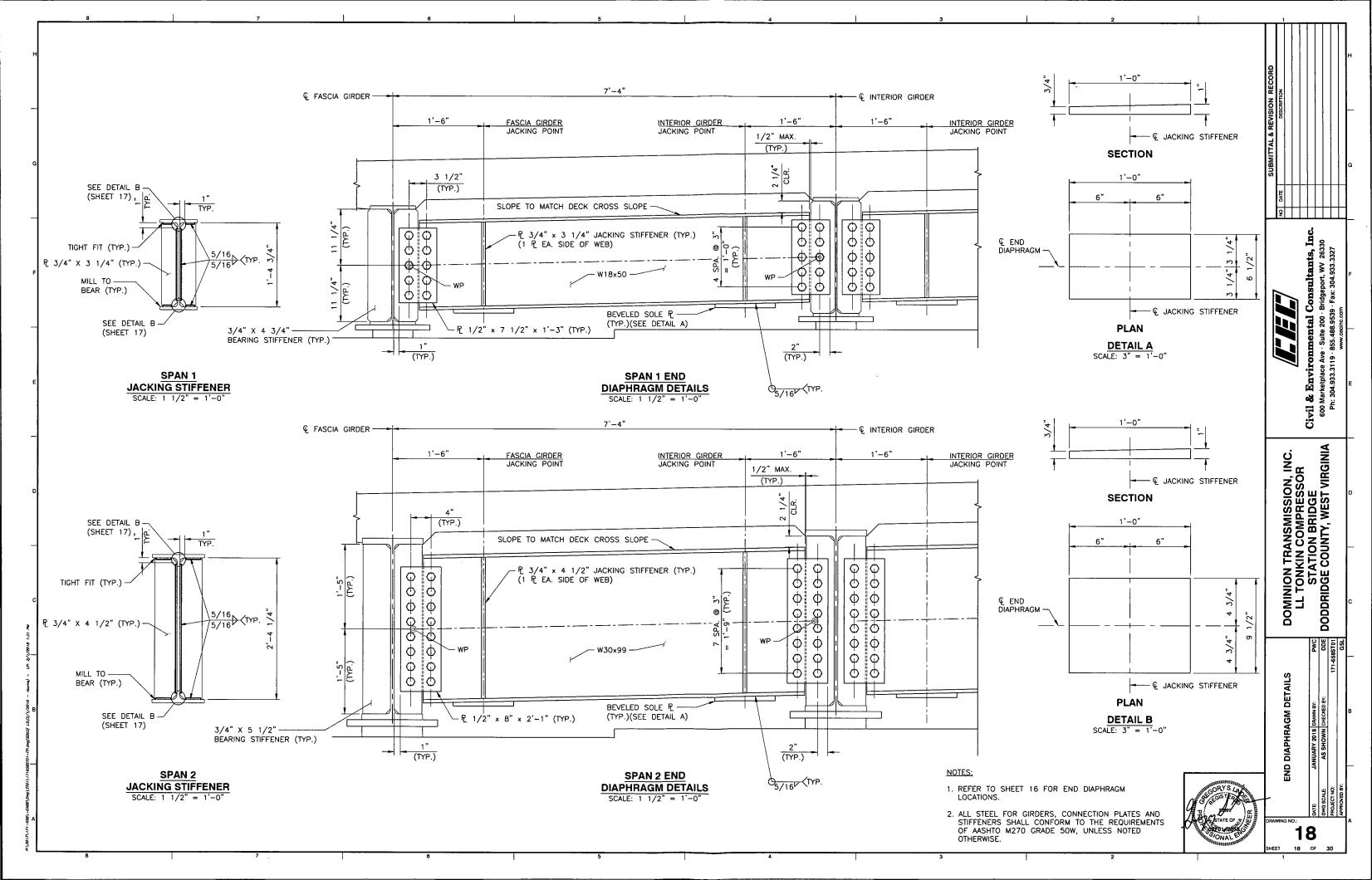
G8

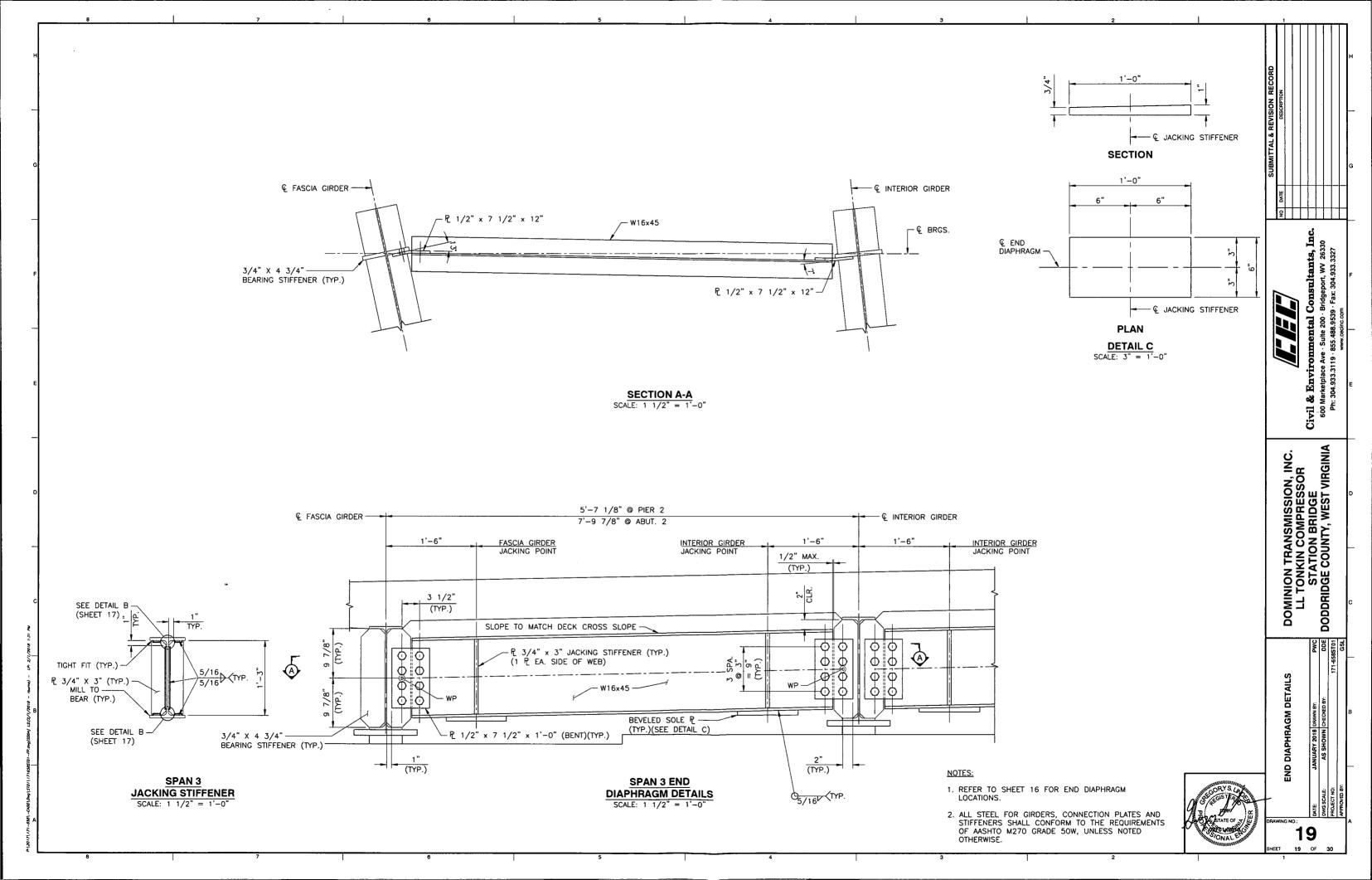
90'00'00"

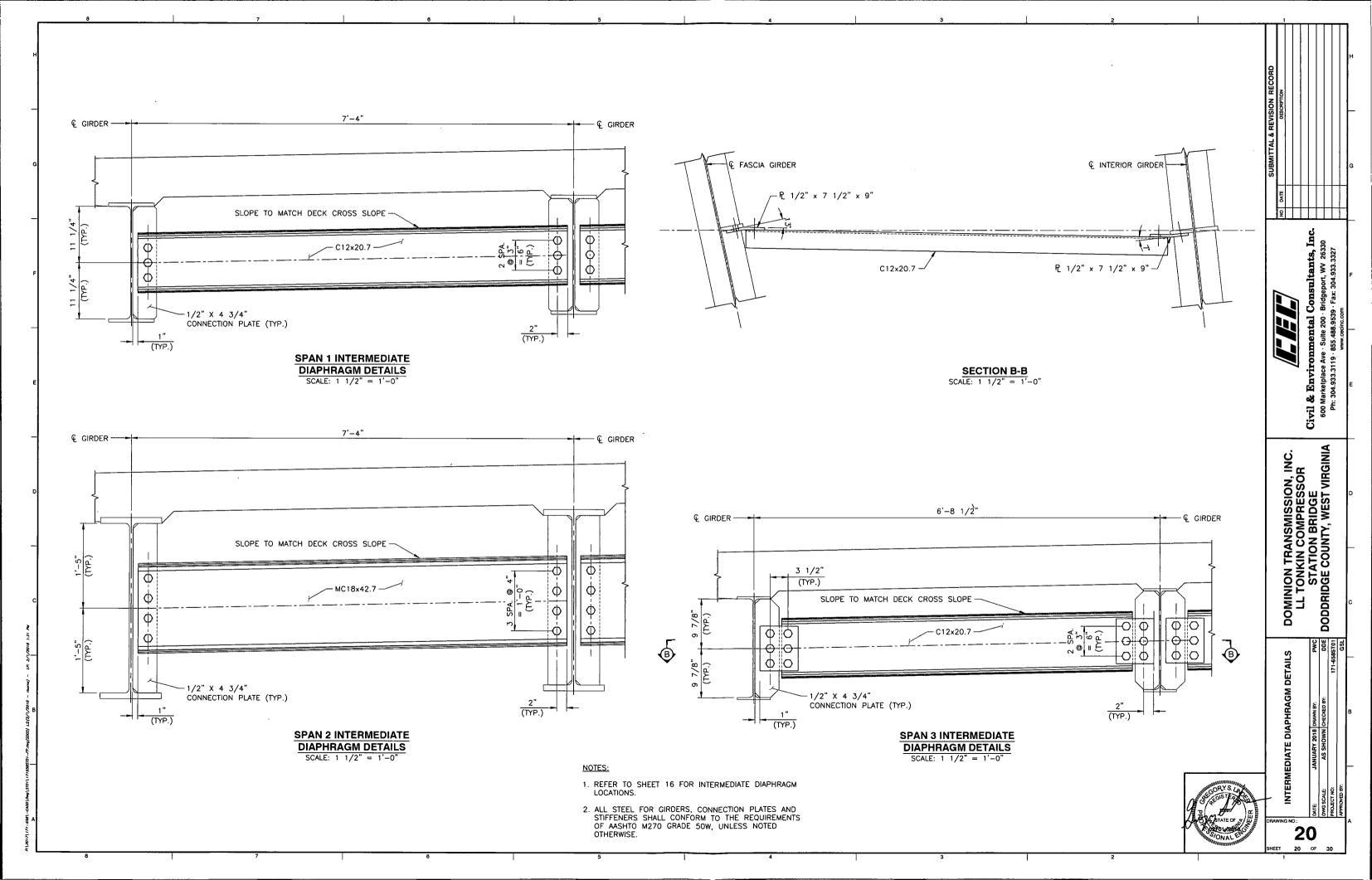
90'00'00"

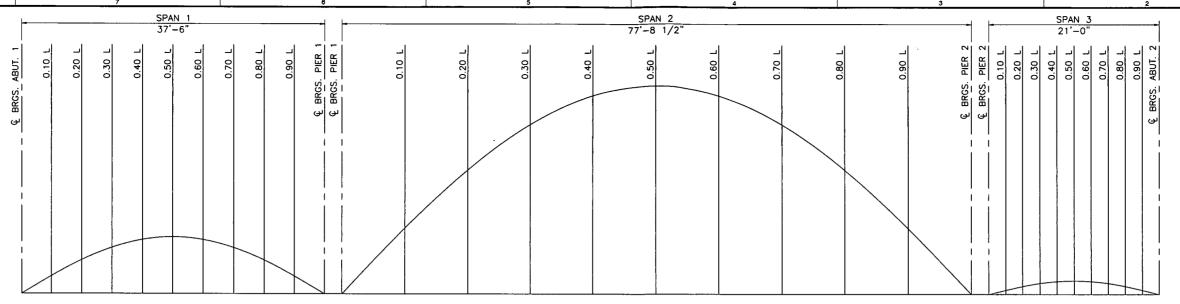












	SPAN 1											SPAN 2													
	DESCRIPTION	0.00 L	0.10 L	0.20 L	0.30 L	0.40 L	0.50 L	0.60 L	0.70 L	0.80 L	0.90 L	1.00 L		DESCRIPTION	0.00 L	0.10 L	0.20 L	0.30 L	0.40 L	0.50 L	0.60 L	0.70 L	0.80 L	0.90 L	1.00 L
	DEFLECTION DUE TO WEIGHT OF STEEL	0.000	0.019			0.057	0.060	0.057	0.049	0.036	0.019	0.000		DEFLECTION DUE TO WEIGHT OF STEEL	0.000	0.157	0.297	0.406	0.476	0.500	0.476	0.406	0.297	0.157	0.000
G1	DEFLECTION DUE TO SLAB DEAD LOAD	0.000	0.165		0.428	0.501	0.526	0.501			0.165	0.000	C5	DEFLECTION DUE TO SLAB DEAD LOAD	0.000	0.580	1.091	1.492	1.747	1.835	1.748	1.492	1.092	0.581	0.000
"	DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD	0.000	0.013		0.034	0.040	0.042	0.040	0.034	0.025	0.013	0.000	65	DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD	0.000	0.065	0.122	0.168	0.196	0.206	0.196	0.168	0.123	0.065	0.000
	TOTAL CAMBER	0.000	0.197	0.373	0.511	0.598	0.628	0.598	0.511	0.373	0.197	0.000		TOTAL CAMBER	0.000	0.802	1.510	2.066	2.419	2.541	2.420	2.066	1.512	0.803	0.000
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			ļ	<u> </u>																					
	DEFLECTION DUE TO WEIGHT OF STEEL	0.000	0.019		0.050	0.059			0.050	0.037	0.019	0.000		DEFLECTION DUE TO WEIGHT OF STEEL	0.000	0.158	0.299	0.409	0.479	0.503	0.479	0.409	0.299	0.158	0.000
G2	DEFLECTION DUE TO SLAB DEAD LOAD	0.000	0.191		0.494	0.578			0.494	0.361	0.191	0.000	CE	DEFLECTION DUE TO SLAB DEAD LOAD	0.000	0.599	1.125	1.536	1.799	1.888	1.799	1.537	1.125	0.600	0.000
"-	DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD	0.000				0.038			0.032	0.024	0.012	0.000	5 66	DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD	0.000	0.064	0.121	0.165	0.193	0.203	0.193	0.165	0.121	0.064	0.000
	TOTAL CAMBER	0.000	0.222	0.422	0.576	0.675	0.709	0.675	0.576	0.422	0.222	0.000		TOTAL CAMBER	0.000	0.821	1.545	2.110	2.471	2.594	2.471	2.111	1.545	0.822	0.000
İ	DEFLECTION DUE TO WEIGHT OF STEEL	0.000	0.019		<del></del>	0.059		0.059	0.050	0.037	0.019	0.000		DEFLECTION DUE TO WEIGHT OF STEEL	0.000	0.158	0.299	0.409	0.479	0.503	0.479	0.409	0.299	0.158	0.000
G3	DEFLECTION DUE TO SLAB DEAD LOAD	0.000	0.191			0.578		0.578	0.494	0.361	0.191	0.000	C7	DEFLECTION DUE TO SLAB DEAD LOAD	0.000	0.599	1.125	1.536	1.799	1.888	1.799	1.537	1.125	0.600	0.000
	DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD	0.000	0.012	0.024	0.032	0.038	0.040	0.038	0.032	0.024	0.012	0.000	07	DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD	0.000	0.064	0.121	0.165	0.193	0.203	0.193	0.165	0.121	0.064	0.000
	TOTAL CAMBER	0.000	0.222	0.422	0.576	0.675	0.709	0.675	0.576	0.422	0.222	0.000		TOTAL CAMBER	0.000	0.821	1.545	2.110	2.471	2.594	2.471	2.111	1.545	0.822	0.000
1	DEFLECTION DUE TO WEIGHT OF STEEL	<del></del>	0.019			0.057					0.019	0.000		DEFLECTION DUE TO WEIGHT OF STEEL	0.000	0.157	0.297	0.406	0.476	0.500	0.476	0.406	0.297	0.157	0.000
G4	DEFLECTION DUE TO SLAB DEAD LOAD	0.000	0.165		0.428	_	0.526		0.428	0.312	0.165	0.000	CB	DEFLECTION DUE TO SLAB DEAD LOAD	0.000	0.580	1.091	1.492	1.747	1.835	1.748	1.492	1.092	0.581	0.000
"	DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD	0.000	0.013		0.034		0.042	0.040	0.034	0.025	0.013	0.000	90	DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD	0.000	0.065	0.122	0.168	0.196	0.206	0.196	0.168	0.123	0.065	0.000
	TOTAL CAMBER	0.000	0.197	0.373	0.511	0.598	0.628	0.598	0.511	0.373	0.197	0.000		TOTAL CAMBER	0.000	0.802	1.510	2.066	2.419	2.541	2.420	2.066	1.512	0.803	0.000
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			SPAN	٧ 3								
	DESCRIPTION	0.00 L	0.10 L	0.20 L	0.30 L	0.40 L	0.50 L	0.60 L	0.70 L	0.80 L	0.90 L	1.00 L
			· ·				-					
	DEFLECTION DUE TO WEIGHT OF STEEL	0.000	0.003	0.006	0.008	0.009	0.009	0.009	0.008	0.006	0.003	0.000
G9	DEFLECTION DUE TO SLAB DEAD LOAD	0.000	0.039	0.075	0.102	0.120	0.126	0.120	0.102	0.075	0.039	0.000
00	DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD	0.000	0.002	0.004	0.005_	0.006	0.006	0.006	0.005	0.004	0.002	0.000
	TOTAL CAMBER	0.000	0.044	0.085	0.115	0.135	0.141	0.135	0.115	0.085	0.044	0.000
										<u> </u>		
	DEFLECTION DUE TO WEIGHT OF STEEL	0.000	0.007	0.006	0.000	0.010	0.010	0.010	0.000	0.000	0.007	0.000
	DEFLECTION DUE TO SLAB DEAD LOAD	0.000	0.003	0.006	0.008	0.010	0.010	0.010	0.008	0.006	0.003	0.000
G10	DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD	0.000	0.046	0.004	0.120	0.006	0.006	0.006	0.120	0.004	0.046	0.000
	TOTAL CAMBER	0.000	0.002	0.004	0.003	0.157	0.006	0.008	0.003	0.004	0.002	0.000
	TOTAL CAMBER	0.000	0.031	0.090	0.133	0.137	0.104	0.137	0.133	0.090	0.051	0.000
		<del>                                     </del>			<del>                                     </del>					<del>                                     </del>		
l	DEFLECTION DUE TO WEIGHT OF STEEL	0.000	0.003	0.006	0.009	0.010	0.011	0.010	0.009	0.006	0.003	0.000
G11	DEFLECTION DUE TO SLAB DEAD LOAD	0.000	0.048	0.091	0.125	0.146	0.153	0.146	0.125	0.091	0.048	0.000
GII	DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD	0.000	0.002	0.004	0.005	0.006	0.006	0.006	0.005	0.004	0.002	0.000
	TOTAL CAMBER	0.000	0.053	0.101	0.139	0.162	0.170	0.162	0.139	0.101	0.053	0.000
						<u> </u>						
	DEFLECTION DUE TO WEIGHT OF STEEL	0.000	0.003	0.006	0.008	0.010	0.010	0.010	0.008	0.006	0.003	0.000
G12	DEFLECTION DUE TO SLAB DEAD LOAD	0.000	0.046	0.088	0.120	0.141	0.148	0.141	0.120	0.088	0.046	0.000
	DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD	0.000	0.002	0.004	0.005	0.006	0.006	0.006	0.005	0.004	0.002	0.000
	TOTAL CAMBER	0.000	0.051	0.098	0.133	0.157	0.164	0.157	0.133	0.098	0.051	0.000
									<u> </u>	ļ		
	DEFLECTION DUE TO WEIGHT OF STEEL	0.000	0.007	0.006	0.000	0.000	0.000	0.000	0.000	0.000	0.007	0.000
	DEFLECTION DUE TO SLAB DEAD LOAD	0.000	0.003	0.006	0.008	0.009	0.009	0.009	0.008	0.006	0.003	0.000
G13	DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD	0.000	0.009	0.073	0.102	0.120	0.126	0.120	0.102	0.075	0.039	0.000
	TOTAL CAMBER	0.000	0.002	0.004	0.005	0.006	0.006	0.006	0.005	0.004	0.002	0.000
ŀ	TOTAL CAMBER	0.000	0.044	0.003	0.115	0.133	0.141	0.133	0.113	0.000	0.044	0.000

# NOTES:

- 1. L = SPAN LENGTH.
- 2. CAMBER ORDINATES ARE GIVEN IN INCHES.
- 3. DECK SLAB DEAD LOAD INCLUDES SLAB AND STAY-IN-PLACE FORMS.
- 4. SUPERIMPOSED DEAD LOAD INCLUDES PARAPETS BUT DOES NOT INCLUDE FUTURE WEARING SURFACE.



DOMINION TRANSMISSION, INC. LL TONKIN COMPRESSOR STATION BRIDGE DODDRIDGE COUNTY, WEST VIRGINIA

Civil & Environmental Consultants, Inc. 600 Marketplace Ave - Suite 200 - Bridgeport, WV 26330 Ph.: 304.933.319 - 855.48959 - Fax: 304.933.3327

CAMBER ORDINATES

21

21 OF 30

LOCATION				BEARING D	IMENSIONS			В	BEVELED LOAD PL	ATE	REAC	TIONS	MAXIMUM	MAXIMUM BRG.
LOC	ATION	L1	W1	t <sub>i</sub>	t <sub>e</sub>	T1	N	L2	W2	T2	DL	LL	DESIGN LOAD	ROTATION (RAD)
CDAN 1	ABUT. 1	8"	8 3/4"	3/8"	1/4"	1.6088"	2	10"	14 3/4"	1"	24 K	45 K	69 K	0.013 *
SPAN 1	PIER 1	8"	8 3/4"	3/8"	1/4"	1.6088"	2	10"	14 3/4"	1"	24 K	45 K	69 K	0.013 *
CDAN O	PIER 1	10"	12"	3/8"	1/4"	2.1034"	3	12"	18"	1 5/16"	52 K	58 K	110 K	0.019 *
SPAN 2	PIER 2	10"	12"	3/8"	1/4"	2.1034"	3	12"	18"	1 5/16"	52 K	58 K	110 K	0.019 *
SPAN 3	PIÉR 2	8"	6 1/2"	3/8"	1/4"	1.6088"	2	10"	12 1/2"	1 1/8"	13 K	39 K	52 K	0.008 *
SPAN 3	ABUT. 2	8"	6 1/2"	3/8"	1/4"	1.6088"	2	10"	12 1/2"	1 1/8"	13 K	39 K	52 K	0.008 *

t; = THICKNESS OF INTERNAL LAYER

te = THICKNESS OF EXTERNAL LAYER

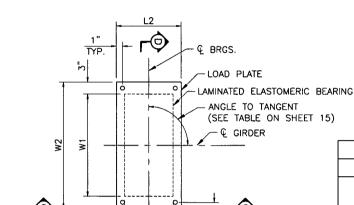
T1 = TOTAL THICKNESS OF ELASTOMERIC BEARING

N = NO. OF BONDED STEEL LAMINATES

T2= LOAD PLATE THICKNESS MEASURED AT & BEARINGS

INTERNAL STEEL LAMINATE THICKNESS = 0.1196" (11 GAUGE) DUROMETER OF ELASTOMER = 60

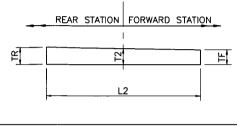
\* INCLUDES 0.005 RADIAN FOR CONSTRUCTION TOLERANCE.



**FIXED BEARING - PLAN** 

SPAN 1-ABUT. 1; SPAN 2-PIER 2; SPAN 3-ABUT. 2

**€** BEARING



€ BEARING

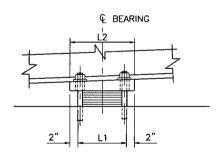
BEVELED LOAD PLATE THICKNESS									
SPAN LOCATION TR TF									
SPAN 1	ABUT. 1	1"	1"						
	PIER 1	1"	1"						
SPAN 2	PIER 1	1 1/2"	1 1/8"						
SPAN Z	PIER 2	1 1/2"	1 1/8"						
60441.7	PIER 2	1 1/2"	3/4"						
SPAN 3	ABUT. 2	1 1/2"	3/4"						

# **BEVELED LOAD PLATE DETAIL**

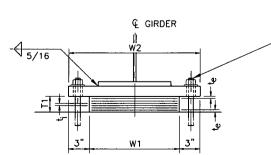
# - € BRGS. -LOAD PLATE - LAMINATED ELASTOMERIC BEARING ANGLE TO TANGENT (SEE TABLE ON SHEET 15) − € GIRDER - SEE DETAIL G **(A)** <u>L</u>

# **EXPANSION BEARING - PLAN**

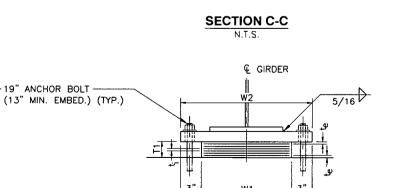
SPAN 1-PIER 1; SPAN 2-PIER 1; SPAN 3-PIER 2



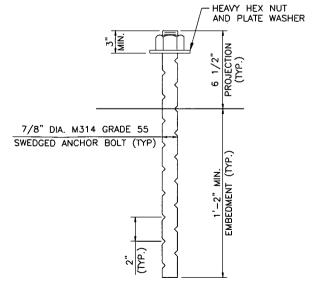
# **SECTION A-A**



**SECTION B-B** 

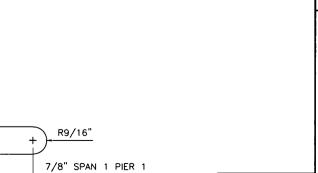


**SECTION D-D** 



**ANCHOR BOLT DETAIL** 

- BEARINGS SHALL BE FABRICATED AND TESTED IN ACCORDANCE WITH AASHTO LRFD BRIDGE CONSTRUCTION SPECIFICATIONS AND THE CONTRACT DOCUMENTS.
- 2. ALL BEARING PADS ARE TO BE MOLDED TO DESIGN DIMENSIONS. CUTTING TO SIZE AFTER FABRICATION IS PROHIBITED.
- 3. THE BEARINGS SHALL BE LOW TEMPERATURE GRADE 3 AND HAVE A SHEAR MODULUS BETWEEN 110-115 PSI, DUROMETER HARDNESS OF 60.
- 4. STEEL LAMINATES SHALL CONFORM TO ASTM 1011 GRADE 60.
- INSTALL BEAMS AND BEARINGS TO ENSURE FULL CONTACT WITH BEARING SURFACES. IF FULL CONTACT IS NOT ACHIEVED AFTER THE DECK IS IN PLACE, FIELD ADJUSTMENTS OR MODIFICATIONS WILL BE MADE BY THE CONTRACTOR TO ENSURE FULL CONTACT AND IS SUBJECT TO THE APPROVAL OF THE ENGINEER.
- 6. THE PROVISIONS FOR THE DESIGN OF THE STEEL—REINFORCED ELASTOMERIC BEARINGS CONFORM TO AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS ARTICL 14.7.5—DESIGN METHOD 8.
- 7. THE STEEL LOAD PLATE SHALL BE AASHTO M270 GRADE 50W. SURFACE PREPERATION SHALL BE INCLUDED IN THE PRICE BID
- 8. PAINT BOTTOM OF STEEL LOAD PLATE WITH EPOXY GRIT (DO NOT
- 9. AFTER THE DECK SLAB HAS BEEN PLACED, JACK AND RESET BEARING ASSEMBLIES AT THE PIERS AND ABUTMENTS TO RELIEVE STRESSES THAT MAY HAVE OCCURRED DURING CONSTRUCTION.
- 10. ANCHOR BOLTS SHALL BE CAST IN PLACE OR PROVIDE 4 INCH DIAMETER CORRUGATED PIPE SLEEVES THAT CAN BE GROUTED



2 1/8" SPAN 2 PIER 1 5/8" SPAN 3 ABUT. 2

**DETAIL G** N.T.S.



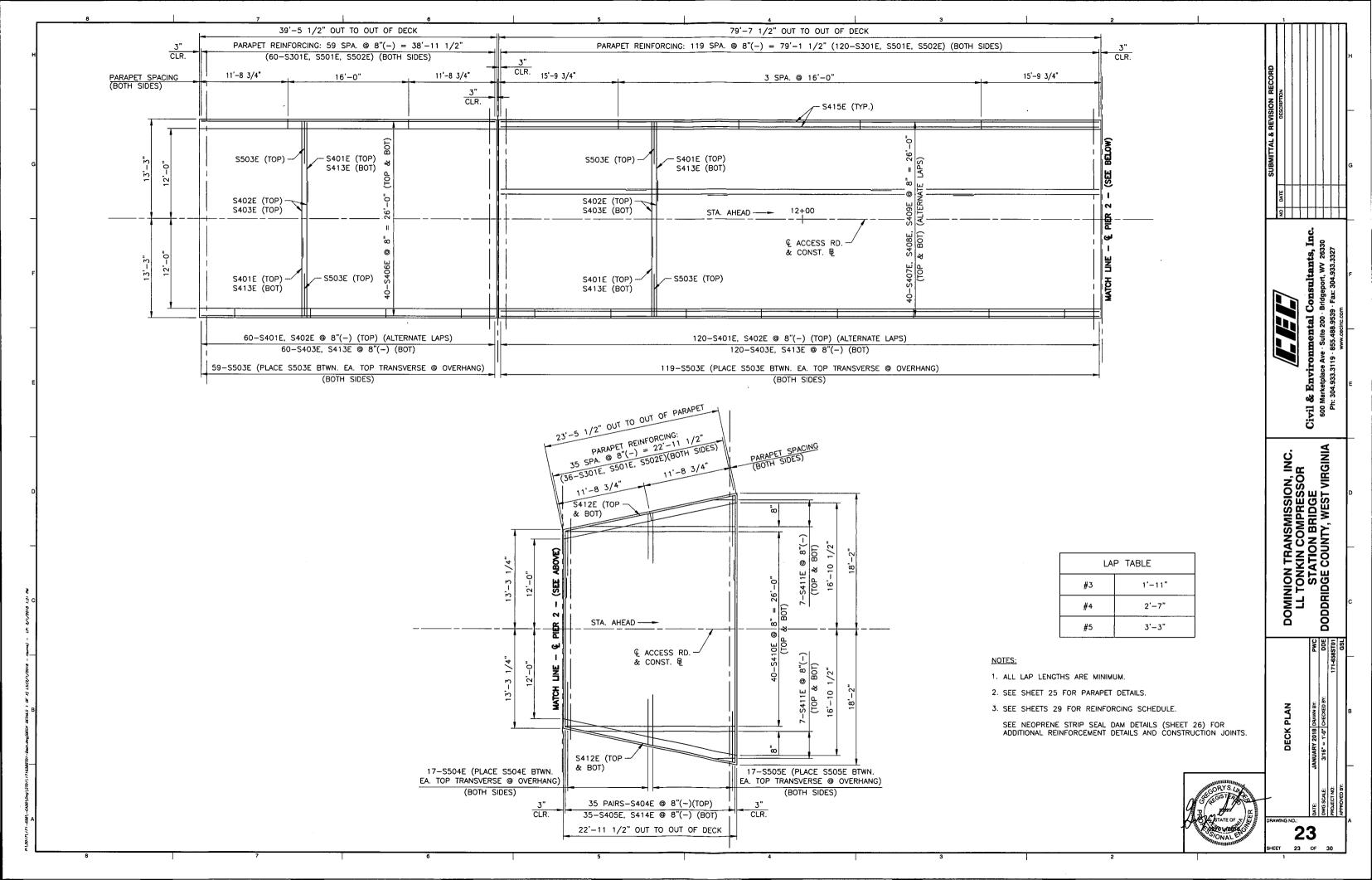
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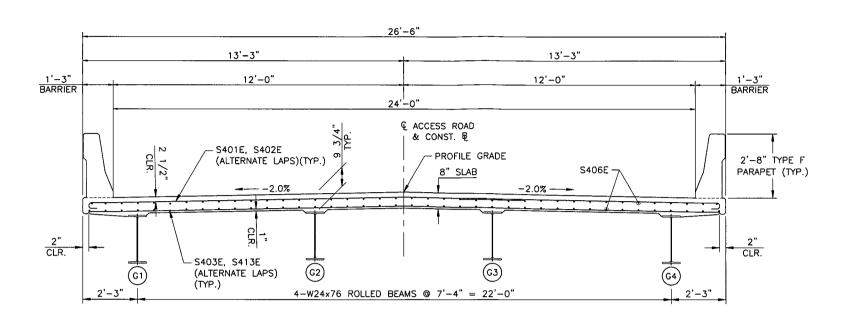
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DOMINION TRANSMISSION, IN LL TONKIN COMPRESSOR STATION BRIDGE DODDRIDGE COUNTY, WEST VIRGII

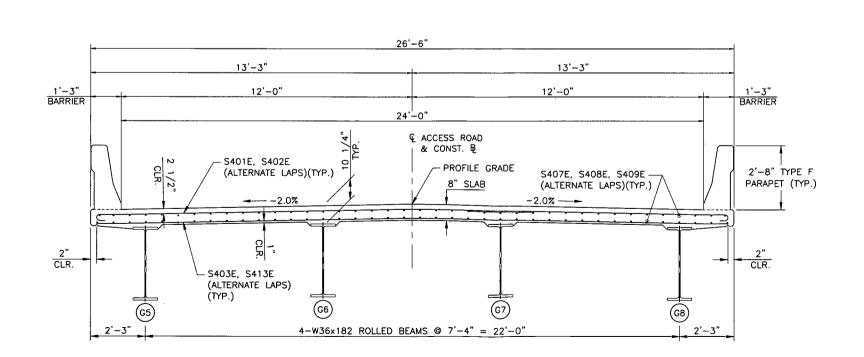
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BEARING DETAILS





# TYPICAL SECTION - SPAN 1 SCALE: 1/2" = 1'-0"



LAP TABLE							
#3	1'-11"						
#4	2'-7"						
<b>#</b> 5	3'-3"						

# NOTES:

- THE TYPE F PARAPETS ARE PLACED WITHOUT VERTICAL JOINTS, BUT ARE VERTICALLY SCORED ALL AROUND WITH 1/4" EDGING TOOL. SEE DECK PLAN ON SHEET 23 FOR SPACING.
- 2. BRACING THE DECK OVERHANG FORMS AGAINST THE EXTERIOR GIRDER MAY PERMIT THE FORMS TO SETTLE AS THE GIRDER DEFLECTS LATERALLY DURING THE CONCRETE PLACEMENT. THE CONTRACTOR SHALL PROVIDE SUFFICIENT BLOCKING TO PREVENT THE
- 3. THE PARAPETS SHALL BE TRULY VERTICAL.
- 4. FOR PARAPET DETAILS, SEE SHEET 25.
- 5. FOR REINFORCING SCHEDULE, SEE SHEET 29



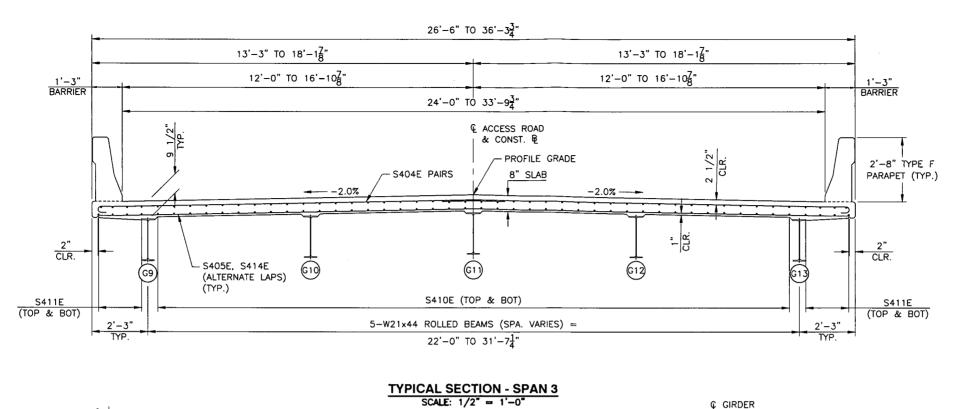
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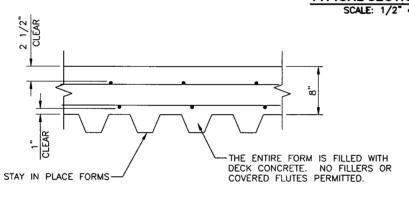
DECK DETAILS

**TYPICAL SECTION - SPAN 2** SCALE: 1/2" = 1'-0"

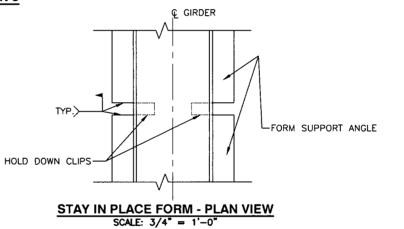


# NOTES:

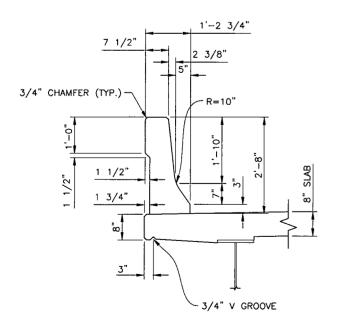
- 1. THE TYPE F PARAPETS ARE PLACED WITHOUT VERTICAL JOINTS, BUT ARE VERTICALLY SCORED ALL AROUND WITH 1/4" EDGING TOOL. SEE DECK PLAN ON SHEET 23 FOR SPACING.
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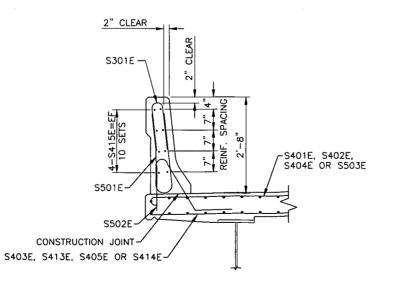
STAY IN PLACE FORM DETAIL
SCALE: 1 1/2" = 1'-0"



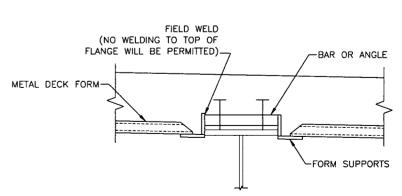
LAP TABLE								
#3	1'-11"							
#4	2'-7"							
<b>#</b> 5	3'-3"							



32" TYPE F PARAPET SCALE: 3/4" = 1'-0"



32" TYPE F PARAPET REINFORCING SCALE: 3/4" = 1'-0"



NOTE: NO WELDING TO TOP OF FLANGE WILL BE PERMITTED

STAY IN PLACE FORM - ELEVATION SCALE: 3/4" = 1'-0"

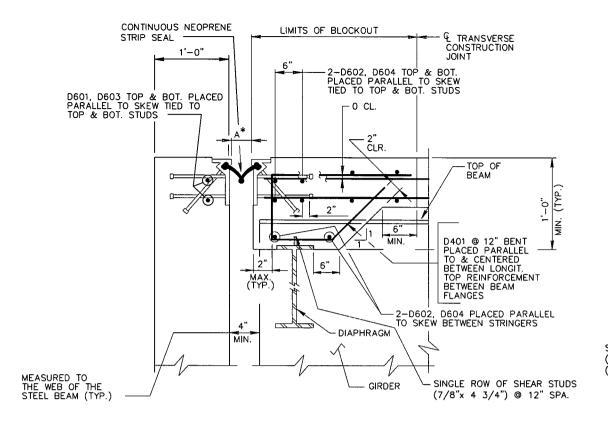


25 OF 30

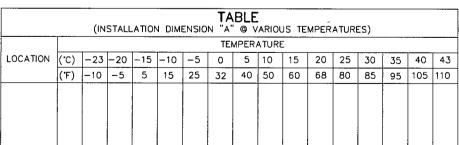
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DECK DETAILS

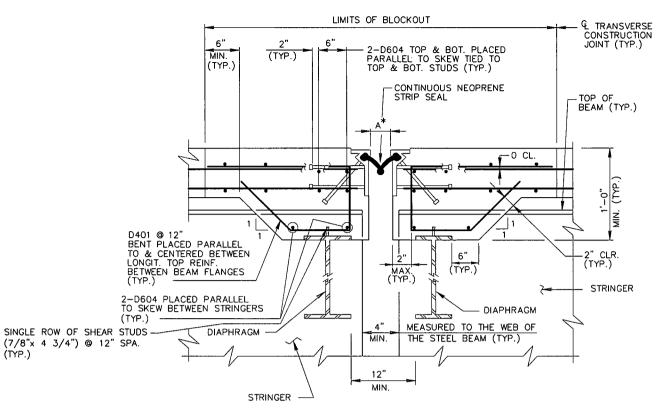


TYPICAL SECTION AT ABUTMENT **FIXED & EXPANSION** 

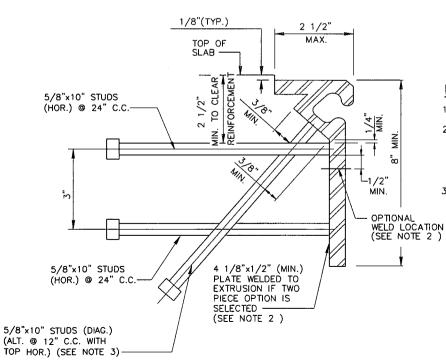


\* - A, IS INSTALLATION WIDTH AS PER TABLE

NOTE: TABLE TO BE SHOWN AND COMPLETED ON FABRICATORS SHOP DRAWINGS.



TYPICAL SECTION AT PIER **FIXED & EXPANSION** N.T.S.



**EXTRUSION SCHEMATIC** N.T.S.

# **EXTRUSION NOTES:**

- 1. EXTRUSION THICKNESS 1/2".
- 2. TWO PIECE MEMBER (EXTRUSION AND PLATE COMBINATION) IN LIEU OF ONE PIECE EXTRUSION IS PERMITTED. WELD IN ACCORDANCE WITH AASHTO/AWS SPECIFICATIONS. (FULL PENETRATION WELD AND N.D.T. REQUIRED)
- 3. STUD SPACING IS 12" MAX. UNLESS SPECIFIED ON AN APPROVED DETAIL.

NEOPRENE STRIP SEAL DAM DETAILS

26

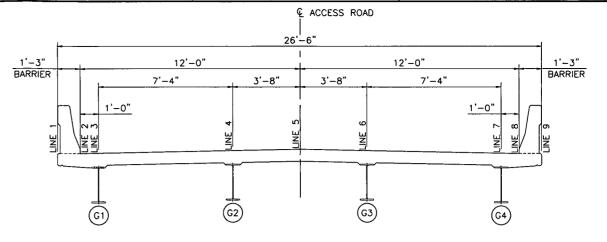
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LL TONKIN COMPRESSOR
STATION BRIDGE
DODDRIDGE COUNTY, WEST VIRGINIA

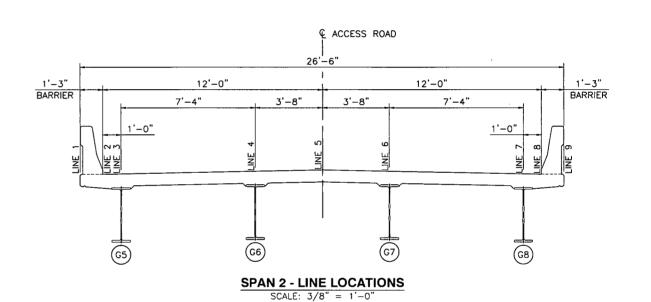
	TABLE OF FINISHED DECK ELEVATIONS												
									LINE 8	LINE 9			
	CL BRG PIER 1	779.93	779.96	779.98	780.12	780.20	780.12	779.98	779.96	779.93			
	0.1	779.70	779.72	779.74	779.89	779.96	779.89	779.74	779.72	779.70			
	0.2	779.47	779.49	779.51	779.66	779.73	779.66	779.51	779.49	779.47			
	0.3	779.23	779.26	779.28	779.42	779.50	779.42	779.28	779.26	779.23			
	0.4	779.00	779.02	779.04	779.19	779.26	779.19	779.04	779.02	779.00			
SPAN 2	0.5	778.77	778.79	778.81	778.96	779.03	778.96	778.81	778.79	778.77			
	0.6	778.53	778.56	778.58	778.73	778.80	778.73	778.58	778.56	778.53			
	0.7	778.30	778.33	778.35	778.49	778.57	778.49	778.35	778.33	778.30			
	0.8	778.07	778.09	778.11	778.26	778.33	778.26	778.11	778.09	778.07			
	0.9	777.83	777.86	777.88	778.03	778.10	778.03	777.88	777.86	777.83			
	CL BRG PIER 2	777.60	777.63	777.65	777.79	777.87	777.79	777.65	777.63	777.60			

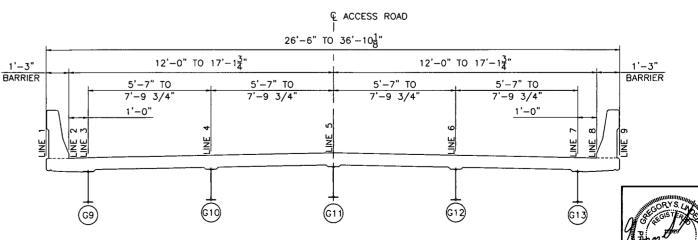
*			TA	BLE OF FINIS	HED DECK E	LEVATIONS				
SPAN NO.	TENTH POINT	LINE 1	LINE 2	LINE 3	LINE 4	LINE 5	LINE 6	LINE 7	LINE 8	LINE 9
	CL BRG PIER 2	777.49	777.52	777.53	777.65	777.76	777.65	777.53	777.52	777.49
	0.1	777.33	777.37	777.38	777.49	777.61	777.49	777.38	777.37	777.33
	0.2	777.18	777.21	777.22	777.34	777.46	777.34	777.22	777.21	777.18
	0.3	777.02	777.05	777.07	777.19	777.32	777.19	777.07	777.05	777.02
	0.4	776.86	776.90	776.91	777.04	777.17	777.04	776.91	776.90	776.86
SPAN 3	0.5	776.71	776.74	776.75	776.89	777.02	776.89	776.75	776.74	776.71
	0.6	776.55	776.58	776.60	776.74	776.88	776.74	776.60	776.58	776.55
	0.7	776.40	776.43	776.44	776.59	776.73	776.59	776.44	776.43	776.40
	0.8	776.24	776.27	776.29	776.43	776.58	776.43	776.29	776.27	776.24
	0.9	776.08	776.11	776.13	776.28	776.43	776.28	776.13	776.11	776.08
	CL BRG ABUT. 2	775.93	775.95	775.98	776.13	776.29	776.13	775.98	775.95	775.93

- ELEVATIONS SHOWN IN THE TABLE ARE FINISHED DECK ELEVATIONS.
- 2. FINISHED DECK ELEVATIONS ARE GIVEN AT TENTH POINTS OF THE CONSTRUCTION CENTERLINE.
- AFTER ALL GIRDERS HAVE BEEN ERECTED AND FALSEWORK REMOVED, THE CONTRACTOR SHALL TAKE ELEVATIONS ALONG THE TOPS OF GIRDERS AT POINTS WHERE THE ELEVATIONS ARE SHOWN IN THE TABLE. THE DIFFERENCE BETWEEN THESE ELEVATIONS AND THE GIVEN TOP OF SLAB ELEVATIONS PLUS THE AMOUNT OF DEFLECTION DUE TO DEAD LOAD OF CONCRETE WILL BE THE THICKNESS OF SLAB OVER THE GIRDERS AT THESE POINTS. THE MINIMUM THICKNESS MEASURED FROM THE TOP OF THE DECK SLAB TO THE TOP OF THE GIRDER WEB AT THE CENTERLINE OF THE GIRDER SHALL NOT BE LESS THAN AS FOLLOWS: 9 3/4" WITHIN SPAN 1, 10 1/4" WITHIN SPAN 2, AND 9 1/2" WITHIN SPAN 3. IF THE GIRDERS HAVE EXCESS CAMBER AND THIS MINIMUM CANNOT BE OBTAINED, THE GRADE LINE SHALL BE ADJUSTED TO OBTAIN THE MINIMUM SLAB THICKNESS.



# **SPAN 1 - LINE LOCATIONS** SCALE: 3/8" = 1'-0"





**SPAN 3 - LINE LOCATIONS** SCALE: 3/8" = 1'-0"

DECK ELEVATIONS

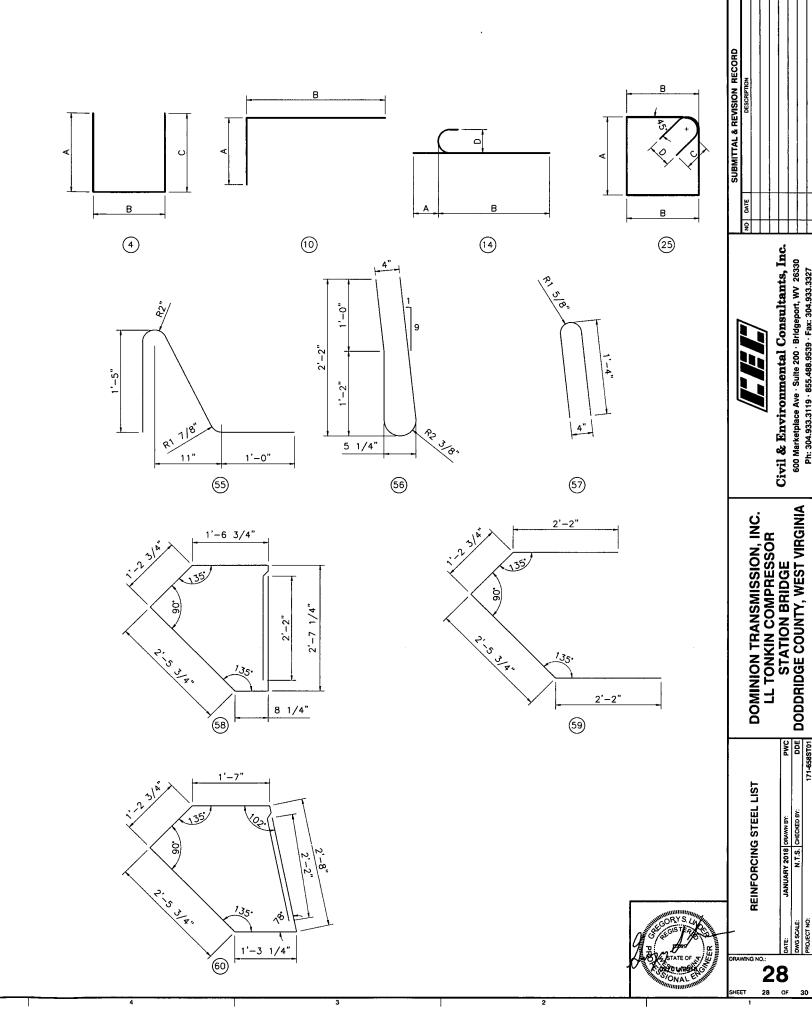
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	·	·-····		ABUT	MENT 1					•	
MARK	SIZE	# of BARS	LENGTU	WEIGHT	TVDE	<u> </u>				INOD	NOTEC
MARK	SIZE	# OI BARS	LENGTH	WEIGHT	TYPE	A	В	С	D	INCR.	NOTES
AB501	5	7	28'-8 "	209	STR					_	-
AB502	5	8	30'-5 "	254	STR						
AB503	5	1	32'-1 5/8"	34	STR						
AB504E	5	4	31'-7 5/8"	132	STR						
AB505E	5	28	9'-0 "	263	4	4'-2"	8"	4'-2"			
AB506	5	6	10'-9 "	67	58						
AB507 AB508E	5 5	14	8'-0 " 30'-5 "	117 127	59 STR					<del></del>	
A0300C	_ ~	<del></del>	30 -3	127	311			<del> </del>		<u> </u>	
AB601	6	28	9'-0 "	379	10	8'-0"	1'-0"				
AB602	6	28	8'-8 "	365	4	3'-0"	2'-8"	3'-0"			
AB603	6	18	11'-6 1/2"	312	STR						
W501	5	16	5'-1 1/2"	86	STR						
W502 W503	5	2	3'-3 1/4" 1'-2 3/4"	7	STR		ļ				
W503	5 5	2 12	9'-2 1/8" TO 11'-6 3/8"	3 130	STR STR	<del> </del>				5 5/8"	2 SETS
W505	5	13	5'-6 "	75	4	2'-2"	1'-2"	2'-2"		3 3/6	2 3513
W506	5	16	5'-5 1/2"	91	STR	1	'-2				
W507	5	2	3'-5 5/8"	7	STR	1				<del></del>	
W508	5	2	1'-3 1/2"	3	STR						
W509	5	14	9'-2 " TO 11'-6 3/8"	151	STR					4 5/8" +	2 SETS
W510	5	2	5'-9 "	12	STR						
W511	5	2	5'-5 1/2"	11	STR						
F501	5	18	30'-1 "	565	STR	-					
F501	5	64	7'-8 "	512	STR	-					
F503	5	40	5'-8 "	237	STR						
F504	5	51	3'-5 "	91	26	3"	2'-8"	6"			
F505	5	14	8'-9 "	128	STR						
F506	5	14	9'-1 "	133	STR						
	L										
				ABUIN	ENT 2	1	<del></del>				1
MARK	SIZE	# of BARS	LENGTH	WEIGHT	TYPE	A	В	С	D	INCR.	NOTES
		,,	22.70177		1					iiiiiiii	140123
AB551	5	9	39'-0 "	366	STR						
AB552	5	10	40'-9 "	425	STR						
AB553	5	1	42'-5 3/4"	44	STR						
AB554E	5	4	41'-11 7/8"	175	STR						
AB555E	5	39	6'-10 "	278	4	3'-1"	8"	3'-1"			
AB556 AB557	5 5	6 18	11'-6 " 8'-0 "	72 150	60 59	<del>                                     </del>					ļ
AB558	5	4	11'-6 "	48	4	2'-0"	7'-6"	2'-0"			
AB559E	5	4	40'-9 "	170	STR	T	, -3				
AB751	7	78	11'-5 "	1,822	10	10'3"	1'-2"				
AB752	7	39	8'-8 "	692	4	3'-0"	2'-8"	3'-0"			
AB753	7	20	13'-5 1/4"	550	STR	<b> </b>					
W451	4	44	12'-11 1/2"	381	STR	-					
W451 W452	4	4	13'-0 "	35	STR	1					
,52	,		15 -0	33	311	1	<del>                                     </del>	-		-	
W651	6	56	12'-4 1/2" TO 13'-4 1/2"	1,084	STR					15/16"	4 SETS
W652	6	28	6'-4 "	267	4	2'-7"	1'-2"	2'-7"			
F551	5	18	40'-5 "	759	STR						
F552	5	84	7'-8 "	672	STR	<del> </del>					
F553 F554	5 5	72 72	5'-11 " 3'-5 "	445 128	STR 26	2"	0' 0"	6"			
F555	5	28	3 -5 16'-7 "	485	STR	3"	2'-8"	6"		+	
333		20	10 - /	700	JIK	+				-	
			·			1	L				

- 1. BAR SIZE IS INDICATED IN THE BAR MARK. FOR EXAMPLE, S501E DENOTES A NO. 5 BAR.
- 2. THE FIRST LETTER OF THE BAR MARK INDICATES THE LOCATION OF THE BAR. FOR EXAMPLE, \$501E INDICATES THE BAR IS SLAB REINFORCEMENT.
- 3. THE BAR MARKS FOR EPOXY-COATED REINFORCEMENT END WITH THE LETTER "E".
- 4. ALL DIMENSIONS ARE OUT TO OUT OF BAR.
- 5. ALL RADII AND DIAMETERS SHOWN ARE INSIDE DIMENSIONS.



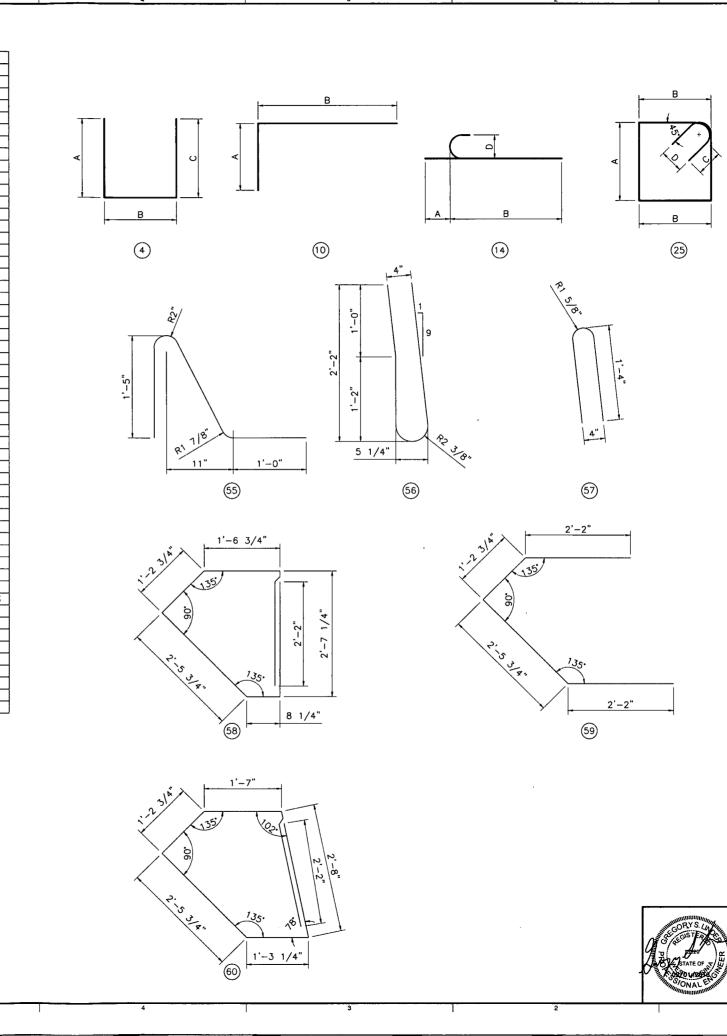
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					PIER 1		<del></del>				
	T			<del></del>	TIER I	·	1			T	
MARK	SIZE	# of BARS	LENGTH	WEIGHT	TYPE	А	В	С	D	INCR.	NOTES
P401	4	3	24'-11 "	50	STR						
P601	6	72	4'-4 "	469	STR						
P602	6	16	14'-3 "	343	25	2'-8"	3'-10"	4 1/2"	4 1/2"		
P603	6	36	9'-1 1/2"	494	4	2'-7 3/4"		2'-7 3/4"			
P604	6	26	7'-4 "	287	4	2'-4"	2'-8"	2'-4"			
P605	6	11	24'-11 "	412	STR						
P801	8	7	24'-11 "	466	STR						
		<u> </u>		<u> </u> 	PIER 2						
MARK	SIZE	# of BARS	LENGTH	WEIGHT	TYPE	A	В	С	D	INCR.	NOTES
P451	4	3	24'-11 "	50	STR	<del> </del>					
P452	4	3	8'-2 1/2"	16	4	1'-6"	5'-2 1/2"	1'-6"			
P651	6	72	4'-4 "	469	STR						
P652	6	16	14'-3 "	343	25	2'-8"	3'-10"	4 1/2"	4 1/2"		
P653	6	36	9'-1 1/2"	494	4	2'-7 3/4"		2'-7 3/4"	7 1/2		
P654	6	26	7'-8 "	300	4	2'-6"	2'-8"	2'-6"			
P655	6	11	24'-11 "	412	STR						
				<u> </u>							
P851	8	7	24'-11 "	466	STR						
					SLAB						
	Т	T		I	3046				1		
MARK	SIZE	# of BARS	LENGTH	WEIGHT	TYPE	A	В	С	D	INCR.	NOTES
S301E	3	248	2'-11 "	272	55						
S401E	4	180	11'-1 1/2"	1,338	14	6"	10'-7 1/2"		3"		
S402E	4	180	18'-5 1/2"	2,220	14	6"	17'-11 1/2"		3"		
S403E	4	180	18'-0 "	2,165	STR	<del>                                     </del>	,,,		_ <u> </u>		
S404E	4	70	37'-6 "	1,754	14	6"	37'-0"		3"		
S405E	4	35	8'-11 " TO 11'-3 "	236	STR					3 3/8"	
S406E	4	80	38'-11 1/2"	2,082	STR						
S407E	4	80	30'-0 "	1,604	STR						
S408E	4	80	35'-0 "	1,871	STR						
S409E	4	80	19'-4 "	1,033	STR						
S410E S411E	4	80 28	22'-5 1/2" 2'-9 3/4" TO 19'-8 "	1,200	STR STR					0' 0 7 /4"	
S411E	4	4	22'-11 1/2"	210 61	STR				ļ	2'-9 3/4"	4 SEIS
S413E	4	180	10'-8 "	1,283	STR	<del> </del>				<del> </del>	
S414E	4	35	19'-11 " TO 21'-11 "	489	STR	<u> </u>				3 3/8"	
S501E	5	248	4'-8 "	1,208	56				ļ		
S501E S502E	5	248	4'-3 "	1,100	55	<del> </del>					
S503E	5	356	6'-3 "	2,323	14	7"	5'-8"		3 3/4"		
S504E	5	17	6'-8 "	118	14	7"	6'-1"		3 3/4"		
S505E	5	17	7′-5 "	132	14	7"	6'-10"		3 3/4"	<del> </del>	

# NOTES:

- 1. BAR SIZE IS INDICATED IN THE BAR MARK. FOR EXAMPLE, S501E DENOTES A NO. 5 BAR.
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REINFORCING STEEL LIST

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